

July 3, 2018 25600-18

Taylor McDaniel 66 Wellington Road 7, Unit 1 PO Box 1156 Elora, ON N0B 1S0

Dear Sir:

Re: Functional Servicing and Stormwater Management Report Proposed Condominium Development 6552 Beatty Line Part of Lot 18, Concession 14 Geographic Township of Nichol Township of Centre Wellington

### **1.0 Introduction**

Van Harten Surveying Inc. was retained by Taylor McDaniel to prepare a functional servicing and stormwater management design and report for the above mentioned property located on the southwest corner of Beatty Line and Farley Road. This work is being done in support of Rezoning, Lot Line Adjustment and Draft Plan of Condominium applications.

This report will summarize the proposed plan as it pertains to site servicing including sanitary, storm, and water supply. This will be done in accordance with the accepted engineering practices and criteria as noted by the local approval agencies, as well as the municipal servicing standards.

### 2.0 Site and Project Description

The project will involve combining the properties known as 6552, 6554, 6556 and 6558 Beatty Road into one property for the purpose of creating one overall condominium plan. Each of these properties currently contain single family dwellings, each individually serviced from Beatty Line. The properties are vegetated with grass and are lightly treed. The properties currently drain towards Beatty Line with a total topographic relief of approximately 4.0m. Water, sanitary and storm servicing is available to the site from Beatty Line. Servicing for these utilities currently exists along Farley Road, however it is noted that it is the preference of the township to avoid servicing the site through Farley Road wherever possible.

Adjacent to the site to the east is a new subdivision currently under construction. At this time it is understood the majority of road and servicing construction to the individual properties within the subdivision is complete, and several homes are either under construction or fully constructed. To the

12 Memorial Avenue423 Woolwich Street71 Weber Street East660 Riddell Road, Unit 1Elmira, ON N3B 2R2Guelph, ON N1H 3X3Kitchener, ON N2H 1C6Orangeville, ON L9W 5G5Phone: 519-669-5070Phone: 519-821-2763Phone: 519-742-8371Phone: 519-940-4110

### www.vanharten.com

R.P. Magahay, B.A. J.E. Buisman, B.Sc., O.L.S. R.M. Mak, B.Sc., O.L.S. J.M. Laws, B.Sc., O.L.S. J.M. Duffy, P.Eng.



south of the subject property are more single family dwellings on large properties, similar in size and condition to the lots to be re-developed.

Referring to the Grading and Servicing Plans attached as Appendix A, the proposal is to construct 16 semi-detached houses (32 dwelling units), one fully detached dwelling, and a four storey apartment building containing 71 dwelling units. Construction of the apartment building will also necessitate the construction of a parking lot with parking for approximately 94 vehicles. A common element roadway will also be constructed with attachments to Beatty Line and Farley Road to access the proposed dwellings.

Following development, it is proposed to extend private sanitary and water servicing through a common element roadway to service the proposed semi-detached and fully detached condominium units. Connection of these services will be to Beatty Line. Servicing for the apartment building will be achieved through an existing sanitary stub located under Farley Road near the intersection of Farley Road and Beatty Line, and a new water service to be constructed at Beatty Line.

Stormwater Management on site will be self-contained and controlled for quantity and quality. Minor stormwater events will be directed underground to connections at Beatty Line where they ultimately outlet to a wetland on the east side of Beatty Line. Major storms will drain as per existing conditions, generally overland towards Beatty Line.

## 3.0 Water Supply

It is understood that each of the existing dwellings as part of the proposed development have a private connection to the water supply located under Beatty Line. As indicated on the Preliminary Grading and Servicing Plan found in Appendix A, all existing water services directed towards the subject property are to be decommissioned in accordance with township standards.

It is proposed to provide water to the proposed condominium development by installing a 150 mm watermain with a connection to the existing 300 mm watermain found under Beatty Line. In order to continually provide a flow of water, it is typically preferred to loop the water system. As water servicing under Farley Road currently exists, it would be preferable to provide a connection to the existing watermain on Farley Road as well, as indicated on the attached plan.

Fire protection on site will be provided by a new private fire hydrant to be installed near the common parking area and Unit 11. Fire hydrants are available near the entrance to the site from both Farley Road and Beatty Line, which will provide the additional required fire protection on site.

Individual units within the condominium plan will be serviced through the proposed watermain to be located in the common elements. In accordance with township and Ontario Building Code (OBC) standards, each unit will be provided with individual 25 mm water services. Each unit will be privately metered.



Available water pressure to service this development should be verified by the township prior to commencement of this development. It is recommended to perform hydrant flow tests during the detailed design stage to determine available water pressures and flows within the existing system.

Water demand to the condominium units in accordance with MOECC design standards and in reference to township servicing standards is approximately 450 L/day per capita. Assuming 35 single family dwelling units and an average of 2.5 occupants per dwelling, a total water demand of 39,375 L/day is calculated. Peaking factors, in accordance with MOECC design standards, are 3.6 as a maximum day factor, and 5.4 for the peak hour factor. Fire flows will be over and above the calculated peak flows.

The water system for the proposed apartment building will be connected to the municipal water supply under Beatty Line. Demand flows for the apartment will be calculated similar to the above, however we will rely on more detailed design from a mechanical consultant where more details regarding fire protection and water pressures at the top floor will be provided.

## 4.0 Sanitary Servicing

It is understood that each of the existing dwellings as part of the proposed development have a private connection to the sanitary sewer located under Beatty Line. As indicated on the Preliminary Grading and Servicing Plan found in Appendix A, all existing sanitary services directed towards the subject property are to be decommissioned in accordance with township standards.

Sanitary servicing to the site will be achieved with a connection to the existing 250 mm sanitary sewer located under Beatty Line with a 200 mm sewer main. Individual condominium units will be connected to this main with typical 100 mm PVC sanitary laterals. Preliminary calculations based on known pipe depths and existing grading would indicate that a gravity connection to the main under Beatty Line is feasible.

Sanitary sewer demand for the proposed condominium units has been calculated based on Township of Centre Wellington municipal servicing standards with per capita flows of 450 L/day per capita and a population of 87.5. Therefore, daily flows of approximately 39,375 L/day are to be expected, with peak flows calculated to be approximately 2.33 L/sec, combining the peaking factor as per Harmon Formula and assumed extraneous flows. Although this is a relatively low demand, the available capacity of the Beatty Line sewer should be verified.

Sanitary servicing to the proposed apartment building will be achieved through an existing 150 mm sanitary stub noted on as-built plan and profile drawings of Farley Road near the intersection of Beatty Line. It is assumed sanitary demand to the apartment building will not exceed the capacity of the existing 150 mm service. Actual demand and available capacity will be verified when mechanical designs of the proposed building are made available.



## 5.0 Stormwater Management

It is assumed all existing dwellings within the development are not serviced with storm connections to Beatty Line. Storm servicing is generally unavailable near the frontage of these dwellings. Therefore, no alteration to existing storm servicing is required to facilitate the proposed development.

An existing stormwater outlet for the property is understood to be near the corner of Beatty Line and Farley Road, or the northeast corner of the subject property. Here, a double inlet catchbasin manhole is constructed with an underground connection to a 1,800 by 900 mm box culvert running diagonally northwest to southeast under Beatty Line. The outlet of this culvert is understood to be into an existing wetland on the opposite side of Beatty Line from the subject property. As this is currently the only suitable underground connection available to the subject property, the minor storm outlet from the site will be to this manhole. Major storms will drain overland to Beatty Line and ultimately outlet to this same wetland.

Preliminary stormwater modelling has been completed using MIDUSS with design storms provided by the Township of Centre Wellington. As noted above, the storm system for the entire site will be directed through an existing catchbasin manhole located at the northeast corner of the site, which is assumed to be the general location of the existing stormwater outlet. Peak flows will be attenuated using parking lot storage, underground storage in superpipes located under the common element and a small dry pond to be located between Unit 8 and Beatty Line. Preliminary feasibility calculations using MIDUSS based on the enclosed grading and servicing plan are attached as Appendix B.

A summary of the peak flow rates and required storage volumes based on preliminary modelling using MIDUSS are listed below:

	Peak Flow Rate (m <sup>3</sup> /sec)					
	Existing Proposed		Total Storage			
	Controlled		Volume			
Return Period			Required (m <sup>3</sup> )			
5-year	0.152	0.146	214.4			
100-year	0.488	0.339	546.1			

## 6.0 Conclusions

The completed servicing and grading design is specific to the subject property and cannot be applied to different properties. It has been determined that municipal servicing exists and is generally suitable for this property, and overland stormwater conveyance is available, where required. It is noted that some further investigation to the available capacity of the existing infrastructure on Beatty Line and Farley Road may be warranted prior to commencement of this development.



I trust that this report and design has been completed within our terms of reference and is suitable for your present requirements. Please contact our office if you have any questions or require further consultation.

Van Harten Surveying Inc.



Mike Vaughan, P. Eng.

Encl. Appendix A – Preliminary Site, Grading and Servicing PlansEncl. Appendix B – MIDUSS Calculations



# Appendix A Preliminary Site, Grading and Servicing Plans









Appendix B MIDUSS Calculations

# 5-Year Storm – Existing Conditions

"		MIDUSS Output				>"
"		MIDUSS version		Ve	ersion 2	.25 rev. 465"
"		MIDUSS created		Tuesda	ay, Febr	uary 05, 2008"
"	10	Units used:				ie METRIC"
"		Job folder:	ς	2:\18-256\2	5600-18	(Beatty Line)"
"		Output filename:				5-yr ex.out"
"		Licensee name:				Mike.Vaughan"
"		Company				"
"		Date & Time last use	ed:	5/24	4/2018 a	t 11:23:24 AM"
"	31 TI	IME PARAMETERS"				
"	5.000	Time Step"				
"	180.000	Max. Storm length"				
"	1500.000	Max. Hydrograph"				
"	47 FI	ILEI_O Read/Open FERGU	JS 5YR STORN	1.stm"		
"	1	1=read/open; 2=write	e/save"			
"	1	1=rainfall; 2=hydrog	graph"			
"	1	1=rain; 2=imperv; 3=	=perv"			
"	FE	ERGUS 5YR STORM.stm"				
"	5	YRS STORM - FERGUS SH	HAND DAM"			
"	Ne	ew storm defined"				
"	Тс	otal depth	48.10	)6 mm"		
"	Ma	aximum intensity	119.48	30 mm/hr'		
"	Du	uration	180.00	0 minutes	s"	
"		0.000 0.000	0.000	0.000 c.m/	/sec"	
"	6	005hyd Hydrograph	extension u	used in this	s file"	
"	33 CA	ATCHMENT 1"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	1	C1 - EXISTING SITE"				
"	5.000	% Impervious"				
"	1.864	Total Area"				
"	152.000	Flow length"				
"	2.000	Overland Slope"				
"	1.771	Pervious Area"				
"	152.000	Pervious length"				
"	2.000	Pervious slope"				
	0.093	Impervious Area"				
	152.000	Impervious length"				
	2.000	Impervious slope"				
	0.250	Pervious Manning 'n'				
	25.000	Pervious Max.infilt	ration"			
	5.000	Pervious Min. Inilit	ration"			
	0.230	Pervious Lag Constan	atemage"			
	0.015	Tervious Depression	storage			
	0.013	Impervious Manning	ltration"			
	0.000	Impervious Min infil	ltration"			
"	0.000	Impervious Lag const	-ant (hours)			
"	1 500	Impervious Depressio	on storade"			
"	1.500			0 000 6	~ m/sec"	
"	Ca	atchment 1	Pervious	Impervious	Total A	rea "
"	S1	irface Area	1.771	0.093	1.864	hectare"
"	Ti	ime of concentration	30.223	5.470	28.100	minutes"
"	Ti	ime to Centroid	121.040	93.101	118.644	minutes"
"	Rá	ainfall depth	48.106	48.106	48.106	mm"
"	Rá	ainfall volume	851.86	44.83	896.69	c.m"
"	Ra	ainfall losses	22.143	1.831	21.128	mm"
"	Rı	unoff depth	25.963	46.275	26.978	mm"
"	Ru	unoff volume	459.75	43.13	502.88	c.m"

"		Runoff	coefficie	ent	0.540		0.962		0.561	"
"		Maximu	um flow		0.146		0.025		0.152	c.m/sec"
"	40	HYDROG	GRAPH Add 1	Runoff	"					
"		4 Ado	d Runoff "							
"			0.152	0.15	20.	000	0	.000"		
"	38	START/	RE-START	TOTALS I	1"					
"		3 Rur	noff Total	s on EX	IT"					
"		Total	Catchment	area				1.	864	hectare"
"		Total	Imperviou	s area				0.	093	hectare"
"		Total	% impervi	ous				5.	000"	
"	19	EXIT"								

# **100-Year Storm – Existing Conditions**

"		MIDUSS Output				>"
"		MIDUSS version		Ve	ersion 2.2	5 rev. 465"
"		MIDUSS created		Tuesda	ay, Februa	ry 05, 2008"
"	10	Units used:				ie METRIC"
"		Job folder:	Ç	2:\18-256\2	5600-18 (B	eatty Line)"
"		Output filename:			10	0-yr ex.out"
"		Licensee name:			M	like.Vaughan"
"		Company				
"		Date & Time last use	ed:	5/24	4/2018 at	11:12:36 AM"
"	31 Т	'IME PARAMETERS"				
"	5.000	Time Step"				
"	180.000	Max. Storm length"				
"	1500.000	Max. Hydrograph"				
"	47 E	ILEI O Read/Open FERG	US 100YR STO	DRM.stm"		
"	1	1=read/open; 2=write	e/save"			
"	1	1=rainfall; 2=hydro	graph"			
"	1	1=rain; 2=imperv; 3=	=perv"			
"	E	ERGUS 100YR STORM.stm	"			
"	F	ergus Shand Dam using	Environment	t Canada IDI	F curve da	ta"
"	Ň	wew storm defined"				
"	Т	otal depth	93.22	24 mm"		
"	M	laximum intensity	211.62	20 mm/hr'		
"	Ľ	Puration	180.00	00 minutes	s"	
"		0.000 0.000	0.000	0.000 c.m.	/sec"	
"	6	100hyd Hydrograph	extension u	used in this	s file"	
"	33 C	ATCHMENT 1"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	1	C1 - EXISTING SITE"				
"	5.000	% Impervious"				
"	1.864	Total Area"				
"	152.000	Flow length"				
"	2.000	Overland Slope"				
"	1.771	Pervious Area"				
"	152.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.093	Impervious Area"				
"	152.000	Impervious length"				
"	2.000	Impervious slope"				
	0.250	Pervious Manning 'n				
	25.000	Pervious Max.infilt:	ration"			
	5.000	Pervious Min.infilt:	ration"			
	0.250	Pervious Lag constan	nt (nours)"			
	5.000	Pervious Depression	storage"			
	0.015	Impervious Manning	· [] · ··			
	0.000	Impervious Max.Inii.	ltration"			
	0.000	Impervious Min. Inii.	ttration			
	1 500	Impervious Lag cons	on storage"	1		
	1.500		on storage	0 000 /	a m/soc"	
"	C	0.400 $0.000$	Pervious	Tmpervious	Total Are	.a "
"		urface Area	1.771	0.093	1.864	hectare"
"	Т	ime of concentration	23.791	4.352	22.544	minutes"
"	± т	ime to Centroid	112.697	89.411	111.203	minutes"
"	- R	ainfall depth	93.224	93.224	93.224	mm"
"	F	ainfall volume	1650.55	86.87	1737.42	c.m"
"	F	ainfall losses	23.402	2.286	22.347	mm "
"	F	unoff depth	69.822	90.939	70.878	mm"
"	F	unoff volume	1236.21	84.74	1320.95	c.m"

"		Runoff	coeffici	ent	0.749	0.	975	0.760	"
"		Maximu	m flow		0.467	0.0	045	0.488	c.m/sec"
"	40	HYDROG	RAPH Add	Runoff	"				
"		4 Add	Runoff "						
"			0.488	0.48	8 0.0	000	0.000"		
"	38	START/	RE-START	TOTALS	1"				
"		3 Run	off Total	s on EX	IT"				
"		Total	Catchment	area			1	.864	hectare"
"		Total	Imperviou	s area			0	.093	hectare"
"		Total	% impervi	ous			5	.000"	
"	19	EXIT"							

# 5-Year Storm – Design Conditions

"			MIDUSS Output				>"
"			MIDUSS version		Ve	ersion 2.25	rev. 465"
"			MIDUSS created		Tuesda	ay, February	7 05 <b>,</b> 2008"
"		10	Units used:				ie METRIC"
"			Job folder:	Ç	Q:\18-256\2	5600-18 (Bea	atty Line)"
"			Output filename:			5-yr DH	ESIGN7.out"
"			Licensee name:			Mil	ke.Vaughan"
"			Company				"
"			Date & Time last use	ed:	5/2	24/2018 at 4	1:34:45 PM"
"	31	TI	ME PARAMETERS"				
"		5.000	Time Step"				
"		180.000	Max. Storm length"				
"		1500.000	Max. Hydrograph"				
"	47	FI	LEI_O Read/Open FERGU	JS 5YR STORN	4.stm"		
"		1	1=read/open; 2=write	e/save"			
"		1	1=rainfall; 2=hydrog	graph"			
"		1	1=rain; 2=imperv; 3=	=perv"			
"		FE	RGUS 5YR STORM.stm"				
"		5	YRS STORM - FERGUS SH	HAND DAM"			
"		Ne	w storm defined"				
"		То	tal depth	48.10	06 mm"		
"		Ma	ximum intensity	119.48	30 mm/hr'		
"		Du	ration	180.00	00 minutes	5"	
		_	0.000 0.000	0.000	0.000 c.m,	/sec"	
		6	005hyd Hydrograph	extension u	used in this	s file"	
	33	CA	TCHMENT 101"				
		1	Triangular SCS"				
		1	Equal length"				
		2	Horton equation"			- <b>!!</b>	
		101	CIUI - WEST PORTION	- SEMI-DETA	ACHED HOUSE:	5	
		64.300	% Impervious"				
		0.485	Total Area"				
		44.000	FIOW length"				
		2.000	Deruieus Area"				
"		0.173	Pervious Area				
		2 000	Pervious slope"				
		0 312	Importations Area"				
		44 000	Impervious length"				
		2 000	Impervious slope"				
"		0 250	Pervious Manning 'n'				
"		25 000	Pervious Max infilt	ration"			
"		5.000	Pervious Min.infilt	ration"			
"		0.250	Pervious Lag constar	nt. (hours)"			
"		5.000	Pervious Depression	storage"			
"		0.015	Impervious Manning	'n'"			
"		0.000	Impervious Max.infil	ltration"			
"		0.000	Impervious Min.infil	ltration"			
"		0.050	Impervious Lag const	ant (hours)	) ''		
"		1.500	Impervious Depressio	on storage"			
"			0.089 0.000	0.000	0.000 0	c.m/sec"	
"		Ca	tchment 101	Pervious	Impervious	Total Area	"
"		Su	rface Area	0.173	0.312	0.485	hectare"
"		Ti	me of concentration	14.365	2.600	5.418	minutes"
"		Ti	me to Centroid	101.719	89.050	92.085	minutes"
"		Ra	infall depth	48.106	48.106	48.106	mm"
"		Ra	infall volume	83.29	150.02	233.31	c.m"
"		Ra	infall losses	22.169	2.386	9.449	mm"
"		Ru	noff depth	25.937	45.720	38.657	mm"
"		Ru	noff volume	44.91	142.58	187.49	c.m"

"		Runoff coeffi	cient	0.539	0.950	0.804	"
"		Maximum flow		0.024	0.084	0.089	c.m/sec"
"	40	HYDROGRAPH Ad	d Runoff	"			,
"	4	Add Runoff	"				
"		0.089	0.08	39 0.0	00 0.0	000"	
"	54	POND DESIGN"					
"	0.089	Current pe	ak flow	c.m/sec	"		
"	0.038	Target out	flow c				
	187 5	Hydrograph	volume	c m"			
	21	Number of	stages"	0.111			
	418 050	Minimum wa	ter level	l metre			
	420 250	) Maximum wa	ter level	l metre			
	418 050	Starting W	ator love	al motr	<u> </u>		
	410.000	) Keen Desig	n Data: 1	$I = True \cdot$	0 = False'		
		Lovol Di	schargo	Volumo"	0 Idibe		
		118 050					
		410.050	0.000	0.000			
		410.100	0.00430	0.5101			
		418.270	0.01529	2.8/1"			
		418.380	0.02090	1.776"			
		418.490	0.02529	15.6/5"			
		418.600	0.02903	26.466"			
		418./10	0.03234	39.000"			
		418.820	0.03534	52.636"			
		418.930	0.03810	66.928"			
		419.040	0.04068	81.518"			
		419.150	0.04310	96.058"			
		419.260	0.04539	110.212"			
		419.370	0.04/58	123.593"			
		419.480	0.04967	135.735"			
		419.590	0.05167	145.839"			
		419.700	0.05360	152.890"			
		419.810	0.05546	157.043"			
		420.000	0.05853	159.043"			
"		420.100	0.06009	159.156"			
"		420.140	0.07912	159.201"			
"		420.250	0.1961	159.326"			
"	1.	WEIRS"					
"		Crest	Weir	Crest	Left	Right"	
"		elevation co	efficie	breadth	sideslope	sideslope"	
"		420.100	0.900	1.500	0.000	0.000"	
"	1.	ORIFICES"					
"		Orifice	Orifice	Orifice	Number of		
"		invert co	efficie	diameter	orifices		
		418.050	0.630	0.1400	1.000		
"	1.	SUPERPIPES	_1"				
	1.	Type l is	Pipe"				
		Downstream	Pipe	Pipe	Pipe	Pipe	Number of"
		Invert	Length	Width	Height	Grade %	Pipes"
		418.050	90.000	1.500	1.500	0.500	1.000"
"		Peak outflow		0	.036 c	.m/sec"	
"		Maximum level		418	.864 me	etre"	
		Maximum stora	ge	58	.339 c	.m"	
"		Centroidal la	g	1	.805 hou	urs"	
		0.089	0.089	0.036	0.000	c.m/sec"	
"	40	HYDROGRAPH Ne	xt link '	•			
"	5	Next link					
"		0.089	0.03	36 0.0	36 0.0	000"	
"	33	CATCHMENT 102	"				
"	1	. Triangular	SCS"				
"	1	. Equal leng	th"				
"	2	Horton equ	ation"				
"	102	C102 - SOU	TH PORTIC	DN - SEMI-	DETACHED I	DWELLINGS"	
"	58.500	) % Impervio	us"				

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"
        0.395
               Total Area"
"
       31.500 Flow length"
...
        2.600 Overland Slope"
...
        0.164 Pervious Area"
...
               Pervious length"
       31.500
...
        2.600
                Pervious slope"
...
        0.231
                Impervious Area"
...
       31.500
                Impervious length"
"
        2.600
                Impervious slope"
...
                Pervious Manning 'n'"
        0.250
...
               Pervious Max.infiltration"
       25.000
        5.000 Pervious Min.infiltration"
...
...
        0.250
               Pervious Lag constant (hours)"
...
        5.000 Pervious Depression storage"
...
        0.015 Impervious Manning 'n'"
"
        0.000 Impervious Max.infiltration"
...
        0.000 Impervious Min.infiltration"
...
        0.050
                Impervious Lag constant (hours)"
...
        1.500
                Impervious Depression storage"
...
                     0.075 0.036 0.036
                                                  0.000 c.m/sec"
...
             Catchment 102
                                    Pervious Impervious Total Area "
                                    0.164
...
                                                          0.395 hectare"
             Surface Area
                                               0.231
                                              1.966
88.066
48.106
"
                                                                      minutes"
             Time of concentration 10.865
                                                          4.511
"
             Time to Centroid
                                    97.451
                                                          90.751
                                                                      minutes"
...
             Rainfall depth
                                    48.106
                                                          48.106
                                                                      mm"
                                              111.16
...
                                    78.86
                                                                      c.m"
             Rainfall volume
                                                          190.02
...
             Rainfall losses
                                              2.206
                                   22.187
                                                          10.498
                                                                      mm"
...
             Runoff depth
                                    25.919
                                              45.900
                                                          37.608
                                                                      mm"
...
             Runoff volume
                                    42.49
                                              106.06
                                                          148.55
                                                                      C.m"
...
             Runoff coefficient
                                   0.539
                                              0.954
                                                          0.782
                                                                      ...
...
                                                                      c.m/sec"
             Maximum flow
                                    0.026
                                              0.064
                                                           0.075
"
             HYDROGRAPH Add Runoff "
 40
...
            4 Add Runoff "
"
                     0.075
                                        0.036
                                                  0.000"
                               0.107
"
            POND DESIGN"
 54
"
        0.107 Current peak flow
                                     c.m/sec"
...
        0.057
                Target outflow c.m/sec"
...
        336.2 Hydrograph volume
                                   C. m"
...
          22.
                Number of stages"
...
      417.800
                Minimum water level
                                       metre"
...
      418.900 Maximum water level
                                       metre"
...
                Starting water level
      417.800
                                       metre"
                Keep Design Data: 1 = True; 0 = False"
"
            0
...
                 Level Discharge Volume"
...
                           0.000
                 417.800
                                     0.000"
...
                417.852
                         0.00137
                                     3.802"
...
                                     8.273"
                417.905
                         0.00522
...
                         0.01046
                417.957
                                     13.261"
...
                418.009
                         0.02343
                                     18.864"
"
                418.062
                         0.03028
                                     25.226"
..
                418.114
                          0.03575
                                     32.124"
..
                          0.04057
                 418.167
                                     39.839"
...
                418.219
                          0.04480
                                     48.106"
...
                418.271
                          0.04867
                                     57.078"
...
                                     66.967"
                418.324
                          0.05231
...
                         0.05566
                                     77.418"
                418.376
...
                                     88.846"
                418.429
                         0.05887
"
                418.481
                         0.06186 100.848"
...
                418.533
                         0.06472 113.645"
...
                418.586 0.06750 127.529"
..
                418.638 0.07012 141.988"
...
                418.690 0.07265 157.298"
...
                418.743
                         0.09014
                                    173.798"
```

"		418.795	0.1320	190.879"			
"		418.848	0.1960	192.560"			
"		418.900	0.2805	192.560"			
"	1.	WEIRS"					
"		Crest	Weir	Crest	Left	Right"	
"	e	elevation c	oefficie	breadth s	sideslope si	deslope"	
"		418.700	0.900	1.000	3.000	3.000"	
"	1.	ORIFICES"					
"		Orifice	Orifice	Orifice N	Number of"		
"		invert c	oefficie	diameter	orifices"		
"		417.800	0.630	0.1950	1.000"		
"	1.	LAYERS"					
"		Bottom	Aspect	Bottom	Top	Average"	
		area	ratio	elevation e	elevation si	deslope"	
		67.676	7.774	417.800	418.800	4.000"	
	P€	eak outflow	-	0.	057 c.m/	sec"	
	Ma	aximum leve	1	418.	400 metr	e"	
	Ma	aximum stor	age	82.	683 c.m"		
	Ce	entroidal l	ag	2.	022 hours		
		0.075	0.107	0.057	0.000 c.	m/sec"	
	40 HY	YDROGRAPH	Combine	34"			
	6	Combine "					
	34	Node #"					
		LEAVING S	T.L.E	0	0.5.7		
	Ma			0.	057 C.m/	sec	
	н	yarograph v			259 C.III"		
	4.0 UN		J U.I	U.U. U.U.	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		
	40 ni	Start - N	lait - Ne	w IIIDulaij			
	2	Start - N		ary 0000	.7 0.057	"	
	33 07	0.07 10.07	1"	0.03	0.057		
	1	Triangula	r scs"				
	1	Equal ler	ath"				
"	2	Horton ec	uation"				
"	201	C201 - NC	RTH PORTI	ON OF APART	MENT PARKIN	G LOT"	
"	67.800	% Impervi	.ous"				
"	0.240	Total Are	a"				
"	28.000	Flow leng	th"				
"	2.000	Overland	Slope"				
"	0.077	Pervious	Area"				
"	28.000	Pervious	length"				
"	2.000	Pervious	slope"				
"	0.163	Imperviou	ls Area"				
"	28.000	Imperviou	s length"				
"	2.000	Imperviou	s slope"				
"	0.250	Pervious	Manning '	n'"			
"	25.000	Pervious	Max.infil	tration"			
	5.000	Pervious	Min.infil	tration"			
	0.250	Pervious	Lag const	ant (hours)	"		
	5.000	Pervious	Depression	n storage"			
	0.015	Imperviou	is Manning	'n'"			
	0.000	Imperviou	s Max.inf	LITRATION"			
	0.000	Imperviou	s Min.inf	LLUTATION"	aa) "		
	0.000	Imperviou	is Lag con	scant (nour	.5)"		
	1.500	Imperviou	o o o	LUN STORAGE	; .7 0 0 5 7	a m/aaa"	
	0.	U.U4	יש U.U 1		U.U5/	C.III/SEC"	"
	Ca	accimient 20 urface Area	1	Pervious	1 TIMPETVIOU	n 240	hoctaro"
	51 m-1	ime of core	entration	10 953	1 982	0.240 3 880	minute"
	L _ m 4	ime to Cont	roid	47 55A	1,902 88 001	9.000 90 095	minutes
"	I I De	ainfall dor	nth	48 106	48 106	48 106	mm"
"	Ra	ainfall vol	11me	37.18	78.28	115.45	C.m"
"	Ra	ainfall los	ses	22.183	2.215	8.645	mm"
	110					<del>-</del>	

"	Runoff d	epth	25.922	45.891	39.461	mm"
"	Runoff v	olume	20.03	74.67	94.71	c.m"
"	Runoff c	pefficient	0.539	0.954	0.820	"
"	Maximum	flow	0.012	0.045	0.049	c.m/sec"
"	40 HYDROGRA	PH Add Runofi	£ "			
"	4 Add R	unoff "				
"		0.049 0.0	0.05	57 0.0	57 <b>"</b>	
"	54 POND DES	IGN"				
"	0.049 Curre	nt peak flow	c.m/sec'	•		
"	0.020 Targe	t outflow	c.m/sec"			
"	94.7 Hydro	graph volume	c.m"			
"	21. Numbe	r of stages"				
"	420.310 Minim	um water leve	el metre'	•		
"	420.660 Maxim	um water leve	el metre'	,		
"	420.310 Start	ing water lev	vel metre	e		
"	0 Keep	Design Data:	1 = True; (	) = False"		
"	Lev	el Discharge	Volume"			
"	420.3	10 0.000	0.000"			
"	420.3	27 0.01914	0.00897"			
"	420.3	45 0.01925	0.07829"			
"	420.3	62 0.01936	0.2567"			
"	420.3	80 0.01948	0.6263"			
"	420.3	97 0.01959	1.202"			
"	420.4	15 0.01970	2.114"			
"	420.4	32 0.01981	3.315"			
"	420.4	50 0.01993	5.010"			
"	420.4	67 0.02003	7.066"			
"	420.4	85 0.02014	9.781"			
"	420.5	0.02026	13.121"			
"	420.5	20 0.02036	16.902"			
"	420.5	38 0.02047	21.634"			
"	420.5	55 0.02058	26.842"			
"	420.5	73 0.02068	33.206"			
"	420.5	90 0.02079	40.069"			
"	420.6	0.02090	48.307"			
"	420.6	25 0.02257	57.054"			
	420.6	43 0.03241	67.407"			
	420.6	60 0.05313	78.265"			
	1. WEIRS			_		
	Cre	st Weir	Crest	Left	Right"	
	elevati	on coefficie	breadth s	sideslope	sideslope"	
	420.6	10 0.900	0.000	50.000	50.000"	
	I. ORIFI	JES"	0			
	Orifi	ce Orifice	Orifice M	Number of"		
	inve.	LL COEIIICIE	ulameter	UTILCES"		
	418.8 1 TAVED	LU U.630	0.0850	T.000"		
	I. LAIER	om Nanaat	Dottom	Ter	Arrows wo "	
	BOLL	oni Aspect	alouation of	loustion	Average	
		2a 1allo	420 210			
	Deak out	JU 4.130	420.310	420.000	37.000 m/coc"	
	Feak Out	lovol	420	.020 C.	tro"	
"	Mayimum	storage	420.	101 ~	010 m"	
"	Centroid	al lag	ے <i>ک</i> 1	638 hou	 rs"	
"	0 0	49 N N N N	0 020 1	0 057	_ m/sec"	
"	0.0 ۵.0 ۲۷۵۵۵۵۵	PH Nevt link	U.UZU	0.057	C.III/ 500	
"	5 Nov+	link "				
"	JINEXL	1 049 0 (	0.00 0 0.00	>0 0 0	57 <b>"</b>	
"	33 Сдтснмем	т 202" О.О	0.02		<i></i>	
"	1 Trian	mular SCS"				
"	1 Equal	length"				
"	2 Horto	n equation"				
"	202 C202	- CENTRAL PAR	RKING LOT AN	ID APARTME	NT BUILDING"	

"	91.500	% Impervious"				
"	0.279	Total Area"				
"	24.000	Flow length"				
"	2.000	Overland Slope"				
"	0.024	Pervious Area"				
"	24.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.255	Impervious Area"				
"	24.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n				
"	25.000	Pervious Max.infilt	ration"			
"	5.000	Pervious Min.infilt	ration"			
"	0.250	Pervious Lag consta	nt (hours)"			
"	5.000	Pervious Depression	storage"			
"	0.015	Impervious Manning	'n'"			
"	0.000	Impervious Max.infi	ltration"			
"	0.000	Impervious Min.infi	ltration"			
"	0.050	Impervious Lag cons	tant (hours)	) "		
"	1.500	Impervious Depressi	on storage"			
"		0.073 0.02	0 0.020	0.057 0	c.m/sec"	
"	Ca	atchment 202	Pervious	Impervious	Total Area	"
"	Si	urface Area	0.024	0.255	0.279	hectare"
"	T	ime of concentration	9.985	1.807	2.211	minutes"
"	T	ime to Centroid	96.336	87.799	88.221	minutes"
"	Ra	ainfall depth	48.106	48.106	48.106	mm"
"	Ra	ainfall volume	11.41	122.81	134.22	c.m"
"	Ra	ainfall losses	22.406	2.184	3,903	mm"
"	Ri	unoff depth	25.700	45.922	44.203	mm"
"	Ri	inoff volume	6.09	117.23	123.33	c.m"
"	Ri	noff coefficient	0.534	0.955	0.919	"
"	Ma	aximum flow	0.004	0.072	0.073	c.m/sec"
"	40 H	YDROGRAPH Add Runoff	"			
"	4	Add Runoff "				
"	-	0.073 0.09	3 0.020	0.057"		
"	54 P(	OND DESIGN"	0 01020	0.007		
"	0.093	Current peak flow	c.m/sec"			
"	0.057	Target outflow c	.m/sec"			
"	218.6	Hydrograph volume	C.m"			
"	210.0	Number of stages"	0.111			
"	420.140	Minimum water level	metre"			
"	420.490	Maximum water level	metre"			
"	420.140	Starting water leve	1 metre"			
"	0	Keep Design Data: 1	= True; 0 =	= False"		
"		Level Discharge	Volume"			
"		420.140 0.000	0.000"			
"		420.158 0.04431	0.00803"			
"		420.175 0.04458	0.05914"			
"		420.193 0.04485	0.2056"			
"		420 210 0 04511	0 4737"			
"		420.228 0.04538	0.9415"			
"		420.245 0.04564	1.599"			
"		420.263 0.04591	2.572"			
"		420.280 0.04616	3.792"			
"		420 298 0 04643	5 452"			
"		420 315 0 04668	7 408"			
"		420 332 0 04693	9 783"			
"		420.350 0.04719	12 801"			
"		420.367 0.04744	16.168"			
"		420.385 0.04770	20 329"			
"		420 402 0 04794	24 861"			
		120.102 0.01/94	21.001			
		420.420 0.04870	30.347"			
"		420.420 0.04820 420.438 0.04845	30.347" 36.575"			

... 420.455 0.06061 43.198" " 420.473 0.1347 51.038" ... 420.490 0.2914 59.260" ... 1. WEIRS" ... Crest Weir Crest Left Right" ... elevation coefficie breadth sideslope sideslope" ... 0.000 380.000 380.000" 420.440 0.900 ... ORIFICES" 1. " Orifice Orifice Orifice Number of" ... invert coefficie diameter orifices" ... 1.000" 418.640 0.630 0.1300 ... LAYERS" 1. ... Bottom Aspect Bottom Тор Average" ... ratio elevation elevation sideslope" area ... 0.000 3.516 420.140 420.490 32.200" " 0.048 c.m/sec" Peak outflow ... Maximum level 420.412 metre" ... 27.806 Maximum storage c.m" ... Centroidal lag 1.619 hours" " 0.073 0.093 0.048 0.057 c.m/sec" HYDROGRAPH Next link " " 40 ... 5 Next link " " 0.073 0.048 0.048 0.057" ... 33 CATCHMENT 203" Triangular SCS" 1 ... 1 Equal length" ... Horton equation" 2 ... 203 C203 - SOUTH PARKING LOT AND REAR OF SOUTH SEMIS" ... 60.500 % Impervious" ... 0.321 Total Area" " 26.000 Flow length" ... 2.000 Overland Slope" ... 0.127 Pervious Area" ... 26.000 Pervious length" ... 2.000 Pervious slope" " 0.194 Impervious Area" ... 26.000 Impervious length" " 2.000 Impervious slope" ... 0.250 Pervious Manning 'n'" ... 25.000 Pervious Max.infiltration" ... 5.000 Pervious Min.infiltration" ... 0.250 Pervious Lag constant (hours)" " 5.000 Pervious Depression storage" ... 0.015 Impervious Manning 'n'" ... 0.000 Impervious Max.infiltration" ... 0.000 Impervious Min.infiltration" ... 0.050 Impervious Lag constant (hours)" ... 1.500 Impervious Depression storage" ... 0.062 0.048 0.048 0.057 c.m/sec" " Catchment 203 Pervious Impervious Total Area " " 0.194 Surface Area 0.127 0.321 hectare" ... Time of concentration 10.476 1.896 4.203 minutes" ±.01 87.944 ... Time to Centroid 96.981 90.374 minutes" ... 48.106 Rainfall depth 48.106 48.106 mm" ... Rainfall volume 93.42 61.00 154.42 c.m" ... Rainfall losses 22.240 2.182 mm" 10.105 ... mm" Runoff depth 25.866 45.924 38.001 " Runoff volume 32.80 89.19 121.98 C.m" ... Runoff coefficient 0.790 ... 0.538 0.955 ... Maximum flow 0.062 c.m/sec" 0.021 0.054 **"** 40 HYDROGRAPH Add Runoff " ... 4 Add Runoff " ... 0.062 0.110 0.048 0.057"

"	54	PC	ND DESIGN	1			
"		0.110	Current	peak flow	c.m/sec	2"	
"		0.079	Target o	outflow	c.m/sec"		
"		343.3	Hydrogra	aph volume	c.m"		
"		23.	Number o	of stages"			
"		419.960	Minimum	water leve	el metre	e"	
"		420.360	Maximum	water leve	el metre	<u></u>	
"		419.960	Starting	g water lev	vel meti	ce"	
"		0	Keep Des	sign Data:	1 = True;	0 = False'	•
"			Level	Discharge	Volume'	,	
"			419.960	0.000	0.000'	,	
"			419.979	0.07337	0.01123'	•	
"			419.998	0.07387	0.08958'	•	
"			420.017	0.07436	0.3026	•	
"			420.036	0.07485	0.7175	•	
"			420.055	0.07533	1.400'	,	
			420 074	0 07581	2 421	•	
"			420 093	0 07629	3 842'	,	
			420.000	0.07676	5 737	,	
			420.112	0.07070	0 170	,	
			420.131	0.07724	11 202	,	
			420.151	0.07017	14 014	,	
			420.169	0.07817	14.914		
			420.189	0.07866	19.616		
			420.208	0.07912	24.918		
			420.227	0.07958	31.089'		
			420.246	0.08003	38.214		
"			420.265	0.08194	46.353	•	
"			420.284	0.1106	55.556	•	
"			420.303	0.1875	65.921'	1	
"			420.322	0.3256	70.033'	•	
"			420.341	0.5364	70.033'	•	
"			420.360	0.8288	70.033'	•	
"			420.379	1.212	70.033'	•	
"		1.	WEIRS"				
"			Crest	Weir	Crest	Left	Right"
"		e	levation	coefficie	breadth	sideslope	sideslope"
"			420.260	0.900	2.000	180.000	180.000"
"		1.	ORIFICES				
"			Orifice	Orifice	Orifice	Number of	1
"			invert	coefficie	diameter	orifices"	1
"			418.460	0.630	0.1680	1.000'	,
"		1.	LAYERS"				
"			Bottom	Aspect	Bottom	Тор	Average"
"			area	ratio	elevation	elevation	sideslope"
"			0.000	1.890	419.960	420.310	35.000"
"		Pe	ak outflo	w	(	).079 с.	m/sec"
"		Ма	ximum lev	vel	420	).203 me	etre"
"		Ма	ximum sto	orage	23	3.501 с.	m"
"		Ce	ntroidal	lag	1	L.562 hou	ırs"
"			0.062	0.110	0.079	0.057	c.m/sec"
"	40	НҮ	DROGRAPH	Next link	"		
"	-	5	Next lir	nk "			
"			0.0	)62 0.0	0.0	0.0	)57"
"	3.3	CA	TCHMENT 3	301"			
"		1	Triangul	ar SCS"			
"		± 1	Equal le	ength"			
"		- 2	Horton c	auation"			
"		2 201	C301 - T		TO PORTION	"	
"		46 400	& Tmport	ious"	LONITON		
"		-0.400	• INPEIV	us			
"		6 000	Flor lor	.ea			
"		10 000	TTOM TEL	Igui I globo"			
		10.000	overiand	a stope.			
		0.0//	rervious	area"			

"	6.000	Pervious length"				
"	10.000	Pervious slope"				
"	0.067	Impervious Area"				
"	6.000	Impervious length"				
"	10.000	Impervious slope"				
"	0.250	Pervious Manning 'n'				
"	25.000	Pervious Max.infiltr	ration"			
"	5.000	Pervious Min.infiltr	ration"			
"	0.250	Pervious Lag constar	nt (hours)"			
"	5.000	Pervious Depression	storage"			
"	0.015	Impervious Manning '	'n'"			
"	0.000	Impervious Max.infil	ltration"			
"	0.000	Impervious Min.infil	ltration"			
"	0.050	Impervious Lag const	ant (hours)	"		
"	1.500	Impervious Depressic	on storage"			
"		0.035 0.079	0.079	0.057 c	c.m/sec"	
"	Ca	tchment 301	Pervious	Impervious	Total Area	"
"	Su	rface Area	0.077	0.067	0.144	hectare"
"	Ti	me of concentration	2.682	0.485	1.391	minutes"
"	Ti	me to Centroid	87.394	86.476	86.854	minutes"
"	Ra	infall depth	48.106	48.106	48.106	mm"
"	Ra	infall volume	37.13	32.14	69.27	c.m"
"	Ra	infall losses	22.622	6.142	14.975	mm"
"	Ru	noff depth	25.484	41.964	33.131	mm"
"	Ru	noff volume	19.67	28.04	47.71	c.m"
"	Ru	noff coefficient	0.530	0.872	0.689	
"	Ma	ximum flow	0.018	0.019	0.035	c.m/sec"
"	40 HY	DROGRAPH Add Runoff "	1			
"	4	Add Runoff "				
"		0.035 0.111	0.079	0.057"		
"	40 HY	DROGRAPH Copy to Outf	flow"			
"	8	Copy to Outflow"				
"		0.035 0.111	0.111	0.057"		
"	40 HY	DROGRAPH Combine	34"			
"	6	Combine "				
"	34	Node #"				
"		LEAVING SITE"				
"						

# **100-Year Storm – Design Conditions**

"			MIDUSS Output				>"
"			MIDUSS version		Ve	ersion 2.25	rev. 465"
"			MIDUSS created		Tuesda	ay, Februar	y 05, 2008"
"		10	Units used:				ie METRIC"
"			Job folder:	Ç	Q:\18-256\2	5600-18 (Bea	atty Line)"
"			Output filename:			100-yr D	ESIGN7.out"
"			Licensee name:			Mil	ke.Vaughan"
"			Company				"
"			Date & Time last us	ed:	5/2	24/2018 at 4	4:30:06 PM"
"	31	TI	ME PARAMETERS"				
"		5.000	Time Step"				
"		180.000	Max. Storm length"				
"		1500.000	Max. Hydrograph"				
"	47	FI	LEI_O Read/Open FERG	US 100YR STO	DRM.stm"		
"		1	1=read/open; 2=write	e/save"			
"		1	1=rainfall; 2=hydro	graph"			
"		1	1=rain; 2=imperv; 3	=perv"			
"		FE	RGUS 100YR STORM.stm	"			
"		Fe	rgus Shand Dam using	Environment	t Canada IDI	F curve data	a"
"		Ne	w storm defined"				
"		То	tal depth	93.22	24 mm"		
"		Ma	ximum intensity	211.62	20 mm/hr'		
"		Du	ration	180.00	00 minutes	s"	
"		_	0.000 0.000	0.000	0.000 c.m,	/sec"	
		6	100hyd Hydrograph	extension u	used in this	s file"	
	33	CA	TCHMENT 101"				
		1	Triangular SCS"				
		1	Equal length"				
		2	Horton equation"	05.VT 55.5		- <b>!</b>	
		101	CIUI - WEST PORTION	- SEMI-DETA	ACHED HOUSE:	5"	
		64.300	% Impervious"				
		0.485	Total Area"				
		44.000	Flow length"				
		2.000	Overland Slope"				
		0.1/3	Pervious Area"				
		44.000	Pervious length				
		2.000	Importuious Stope				
		11 000	Impervious Area				
		2 000	Impervious slopo"				
"		2.000	Pervious Manning 'n				
		25 000	Pervious Max infilt	ration"			
"		5 000	Pervious Min infilt	ration"			
"		0 250	Pervious Lag consta	nt (hours)"			
"		5 000	Pervious Depression	storage"			
"		0 015	Impervious Manning	'n'"			
"		0 000	Impervious Max infi	ltration"			
"		0.000	Impervious Min.infi	ltration"			
"		0.050	Impervious Lag cons	tant (hours)	, "		
"		1.500	Impervious Depressio	on storage"			
"			0.200 0.00	0 0.000	0.000	c.m/sec"	
"		Ca	tchment 101	Pervious	Impervious	Total Area	"
"		Su	rface Area	0.173	0.312	0.485	hectare"
"		Ti	me of concentration	11.308	2.068	4.840	minutes"
"		Ti	me to Centroid	97.719	86.237	89.681	minutes"
"		Ra	infall depth	93.224	93.224	93.224	mm"
"		Ra	infall volume	161.41	290.72	452.14	c.m"
"		Ra	infall losses	23.500	2.877	10.239	mm"
"		Ru	noff depth	69.725	90.347	82.985	mm"
"		Ru	noff volume	120.72	281.75	402.48	c.m"

"		Runoff coefficient	0.748	0.969	0.890	"
"		Maximum flow	0.068	0.158	0.200	c.m/sec"
"	40	HYDROGRAPH Add Run	off "			
"	4	Add Runoff "				
"		0.200	0.200 0.	000 0.0	000"	
"	54	POND DESIGN"				
"	0.200	Current peak fl	ow c.m/se	c"		
"	0.038	Target outflow	c.m/sec"			
"	402.5	Hydrograph volu	me c.m"			
"	21.	Number of stage	s"			
"	418.050	Minimum water l	evel metr	e"		
"	420.250	Maximum water l	evel metr	e"		
"	418.050	Starting water	level met	re"		
"	0	Keep Design Dat	a: 1 = True;	0 = False"	1	
"		Level Dischar	ge Volume			
"		418.050 0.0	00 0.000			
"		418.160 0.004	38 0.5161			
"		418.270 0.015	29 2.871			
"		418.380 0.020	90 7.776	"		
"		418.490 0.025	29 15.675	"		
		418.600 0.029	03 26.466			
		418.710 0.032	34 39.000			
		418.820 0.035	34 52.636			
		418.930 0.038	10 66.928			
		419.040 0.040	68 81.518			
		419.150 0.043	10 96.058			
		419.260 0.045	39 IIU.ZIZ			
		419.370 0.047	JO 125.J95			
		419.400 0.049	67 1/5 920			
		419.390 0.031	60 152 890			
		419 810 0.055	46 157 043	n		
		420 000 0 058	53 159 043	n		
		420 100 0 060	09 159 156			
		420 140 0 079	12 159 201			
"		420.250 0.19	61 159.326			
"	1.	WEIRS"				
"		Crest We	ir Crest	Left	Right"	
"		elevation coeffic	ie breadth	sideslope	sideslope"	
"		420.100 0.9	00 1.500	0.000	0.000"	
"	1.	ORIFICES"				
"		Orifice Orifi	ce Orifice	Number of"	,	
"		invert coeffic	ie diameter	orifices"	,	
"		418.050 0.6	30 0.1400	1.000"	,	
"	1.	SUPERPIPES_1"				
"	1.	Type 1 is Pipe"				
"		Downstream Pi	pe Pipe	Pipe	Pipe	Number of"
		Invert Leng	th Width	Height	Grade %	Pipes"
		418.050 90.0	1.500	1.500	0.500	1.000"
		Peak outflow	10	0.116 c.	.m/sec"	
		Maximum level	42	0.191 me	etre"	
		Maximum storage	15	9.259 C.	. m ''	
		Centroidal lag	00 0 110	2.UI3 NOU	11'S"	
	4.0		UU U.II6	0.000	C.III/SeC"	
	4U E	Novt line "	.11K			
	5	NEXL IIIK "	0 116 0	116 0 0	00"	
	33	U.2UU CATCUMENT 102"	0.110 0.	1.0 0.0	000	
	1	Triangular CCC"				
	1	Equal length"				
	2	Horton equation	"			
"	102	C102 - SOUTH PO	RTION - SEMT	-DETACHED F	WELLINGS"	
"	58.500	% Impervious"	02111			

```
"
        0.395
               Total Area"
"
       31.500 Flow length"
...
        2.600 Overland Slope"
...
        0.164 Pervious Area"
...
       31.500
               Pervious length"
...
        2.600
               Pervious slope"
...
        0.231
               Impervious Area"
...
       31.500
                Impervious length"
"
        2.600
                Impervious slope"
...
                Pervious Manning 'n'"
        0.250
...
               Pervious Max.infiltration"
       25.000
        5.000 Pervious Min.infiltration"
...
...
        0.250 Pervious Lag constant (hours)"
...
        5.000 Pervious Depression storage"
...
        0.015 Impervious Manning 'n'"
"
        0.000 Impervious Max.infiltration"
...
        0.000 Impervious Min.infiltration"
...
        0.050
                Impervious Lag constant (hours)"
...
        1.500
                Impervious Depression storage"
...
                     0.169 0.116 0.116
                                                  0.000 c.m/sec"
...
             Catchment 102
                                    Pervious Impervious Total Area "
                                    0.164
...
                                                         0.395 hectare"
             Surface Area
                                               0.231
                                              1.564
85.510
"
                                                                     minutes"
             Time of concentration 8.553
                                                          4.033
                                94.404
"
             Time to Centroid
                                                         88.652
                                                                     minutes"
                                             93.224
...
             Rainfall depth
                                    93.224
                                                         93.224
                                                                     mm"
                                             215.42
...
                                   152.82
                                                                     c.m"
             Rainfall volume
                                                         368.24
...
             Rainfall losses
                                   23.568
                                              2.751
                                                         11.390
                                                                     mm"
...
             Runoff depth
                                   69.656
                                              90.473
                                                         81.834
                                                                     mm"
...
                                   114.18
             Runoff volume
                                              209.06
                                                         323.25
                                                                     C.m"
...
             Runoff coefficient
                                   0.747
                                              0.970
                                                         0.878
                                                                     ...
...
                                                                     c.m/sec"
             Maximum flow
                                    0.071
                                              0.121
                                                          0.169
" 40
             HYDROGRAPH Add Runoff "
...
            4 Add Runoff "
"
                     0.169
                                       0.116 0.000"
                               0.213
"
 54
            POND DESIGN"
"
        0.213 Current peak flow
                                   c.m/sec"
...
        0.057
                Target outflow c.m/sec"
...
        725.2 Hydrograph volume c.m"
...
          22.
                Number of stages"
...
      417.800
               Minimum water level
                                       metre"
..
      418.900 Maximum water level
                                      metre"
...
               Starting water level
      417.800
                                       metre"
                Keep Design Data: 1 = True; 0 = False"
"
            0
...
                 Level Discharge Volume"
...
                417.800
                           0.000
                                     0.000"
..
                417.852
                         0.00137
                                     3.802"
...
                417.905
                         0.00522
                                     8.273"
..
                         0.01046
                417.957
                                    13.261"
...
                418.009
                         0.02343
                                    18.864"
"
                418.062 0.03028
                                    25.226"
"
                418.114
                          0.03575
                                     32.124"
..
                          0.04057
                418.167
                                     39.839"
...
                418.219
                          0.04480
                                     48.106"
...
                418.271
                          0.04867
                                    57.078"
...
                                     66.967"
                418.324
                         0.05231
..
                         0.05566
                                     77.418"
                418.376
...
                                    88.846"
                418.429
                         0.05887
"
                418.481 0.06186 100.848"
...
                418.533 0.06472 113.645"
...
                418.586 0.06750 127.529"
..
                418.638 0.07012 141.988"
...
                418.690 0.07265 157.298"
..
                418.743 0.09014
                                    173.798"
```

"	4	18.795 0.1320	190.879"			
"	4	18.848 0.1960	192.560"			
"	4	18.900 0.2805	192.560"			
"	1. W	VEIRS"				
"		Crest Weir	Crest	Left	Right"	
"	ele	evation coefficie	breadth si	deslope sid	eslope"	
"	4	18.700 0.900	1.000	3.000	3.000"	
"	1. 0	DRIFICES"				
"	0	Drifice Orifice	Orifice Nu	mber of"		
"		invert coefficie	diameter o	rifices"		
"	4	17.800 0.630	0.1950	1.000"		
"	1. L	LAYERS"				
"		Bottom Aspect	Bottom	Тор А	verage"	
"		area ratio	elevation el	evation sid	eslope"	
"		67.676 7.774	417.800	418.800	4.000"	
"	Peak	c outflow	0.1	47 c.m/s	ec"	
"	Maxi	.mum level	418.8	41 metre		
"	Maxi	.mum storage	192.3	29 c.m"		
"	Cent	roidal lag	2.2	34 hours"		
		0.169 0.213	0.147	0.000 c.m	/sec"	
	40 HYDR	ROGRAPH Combine	34"			
	6 C	Combine "				
	34 N	lode #"				
		LEAVING SITE"	0 1	47		
	Maxı	mum flow	0.1	4/ c.m/s	ec"	
	Hyar	ograph volume	/20.5	01 C.M"		
	4.0 11000	U.169 U.2	213 U.14/	0.14/"		
	40 HIDR	KOGRAPH Start - Ne	ew Tribulary"			
	2 5	o 160 - O (	Lary" 200 0 147	0 1 4 7 1		
	22 САЩС	0.109 0.10 10.109 U.V	0.14/	0.14/		
	35 CAIC 1 m	riangular SCS"				
	1 1	'aual length"				
	т <u>п</u> 2 н	Jorton equation"				
	201 0	201 - NORTH PORT.	ION OF APARTM	ENT PARKING	Т.О <b>Т</b> "	
	67 800 %	Tmpervious"	ION OF ALANIE			
	0.240 T	otal Area"				
"	28.000 F	flow length"				
"	2.000 0	)verland Slope"				
"	0.077 P	Pervious Area"				
"	28.000 P	Pervious length"				
"	2.000 P	Pervious slope"				
"	0.163 I	Impervious Area"				
"	28.000 I	mpervious length	"			
"	2.000 I	mpervious slope"				
"	0.250 P	Pervious Manning	'n'"			
"	25.000 P	Pervious Max.infi	ltration"			
"	5.000 P	Pervious Min.infi	ltration"			
"	0.250 P	Pervious Lag const	tant (hours)"			
"	5.000 P	Pervious Depression	on storage"			
"	0.015 I	mpervious Manning	g 'n'"			
	0.000 I	Impervious Max.in:	filtration"			
	0.000 I	mpervious Min.in	tiltration"			
	U.050 I	mpervious Lag com	nstant (hours	) ''		
	1.500 I	mpervious Depres	sion storage"		/	
	~ .	0.105 0.0	JUU 0.147	0.147	c.m/sec"	
	Cato	cnment 201	Pervious	Impervious	Total Area	
	Surt	ace Area	0.0//	U.163	0.240	nectare"
	Time	e of concentration	04 400	T.J//	J.40J 07 024	minutes"
	Time	e lo centrola	94.48∠ 03.224	03.320	01.924	mruluces"
	Raln Doin	fall volumo	JJ.∠∠4 72 ∩1	22.224 151 60	22.224 223 71	
	Ralli	fall lossos	12.04 23 551	2 749	22J.14 9 118	
	Ralli	1707T TOSSES	20.004	L • 179	J. IIO	11111

"	Runo:	ff depth	69.670	90.476	83.776	mm"
"	Runo:	ff volume	53.84	147.22	201.06	c.m"
"	Runo:	ff coefficient	0.747	0.971	0.899	"
"	Maxir	num flow	0.034	0.085	0.105	c.m/sec"
"	40 HYDRO	OGRAPH Add Runof	ff "			
"	4 Ac	dd Runoff "				
"		0.105 0.	.105 0.147	0.147"		
"	54 POND	DESIGN"				
"	0.105 Ci	urrent peak flow	v c.m/sec"			
"	0.020 Ta	arget outflow	c.m/sec"			
"	201.1 H	ydrograph volume	e c.m"			
"	21. Nu	umber of stages'	,			
"	420.310 M:	inimum water lev	vel metre"			
"	420.660 Ma	aximum water lev	vel metre"			
"	420.310 St	tarting water le	evel metre'	1		
"	0 Ke	eep Design Data:	: 1 = True; 0	= False"		
"		Level Discharge	e Volume"			
"	42	20.310 0.000	0.000"			
"	42	20.327 0.01914	1 0.00897"			
"	42	20.345 0.01925	5 0.07829"			
"	42	20.362 0.01936	6 0.2567 <b>"</b>			
"	42	20.380 0.01948	3 0.6263"			
"	42	20.397 0.01959	9 1.202"			
"	42	20.415 0.01970	2.114"			
"	42	20.432 0.01981	L 3.315"			
"	42	20.450 0.01993	3 5.010"			
"	42	20.467 0.02003	3 7.066"			
"	42	20.485 0.02014	9.781"			
"	42	20.503 0.02026	5 13.121"			
"	42	20.520 0.02036	5 16.902"			
"	42	20.538 0.02047	7 21.634"			
"	42	20.555 0.02058	3 26.842"			
"	42	20.573 0.02068	3 33.206"			
"	42	20.590 0.02079	9 40.069"			
"	42	20.608 0.02090	) 48.307"			
"	42	20.625 0.02257	7 57.054"			
	42	20.643 0.03241	67.407"			
	42	20.660 0.05313	3 78.265"			
	1. WI	EIRS"				
		Crest Wein	c Crest	Left	Right"	
	eler	vation coefficie	e breadth si	deslope sic	leslope"	
	42	20.610 0.900	0.000	50.000	50.000"	
	1. 01	RIFICES"				
	01	rifice Orifice	e Orifice Nu	imber of"		
		invert coefficie	e diameter c	rifices"		
	4.	18.810 U.630	0.0850	1.000**		
	1. 14	AILKS" Dottom langet	- Dottom			
		area ratio	s olowation ol	oution sid	lverage	
			) elevation el	ADD GED		
	Pople	0.000 4.130	0 420.310	420.000	37.000	
	reak	Journal Control	120 6	55 motro	."	
"	Mawir	num rever	420.0 75 1	91 c.m."		
"	Max11	num Scoraye	ر ، ر ۱ ډ			
"	Cent.	0 105 0 105	τ·C 5 Ο Ο Λ 7	0 147 ~ ~	/sec"	
"	A0 uvo	OCRAPH Nov+ 1401	, U.U47	0.14/ 0.11	1, 360	
"		ost link "	2			
	5 100	0 105 0	047 0 047	0 1 4 7 "		
"	33 ೧ಶಗಂಗ	HMENT 202"		0.14/		
"		riangular SCS"				
"	1 E	mual length"				
"	2 H	orton equation"				
"	202 C2	202 - CENTRAL PA	ARKING LOT ANI	) APARTMENT	BUILDING"	

"	91.500 % Impervious"				
"	0.279 Total Area"				
"	24.000 Flow length"				
"	2.000 Overland Slope"				
"	0.024 Pervious Area"				
"	24.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.255 Impervious Area"				
"	24.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n	L'''			
"	25.000 Pervious Max.infilt	ration"			
"	5.000 Pervious Min.infilt	ration"			
"	0.250 Pervious Lag consta	int (hours)"			
"	5.000 Pervious Depression	storage"			
"	0.015 Impervious Manning	'n'"			
"	0.000 Impervious Max.infi	ltration"			
"	0.000 Impervious Min.infi	ltration"			
"	0.050 Impervious Lag cons	tant (hours)	) "		
"	1.500 Impervious Depressi	on storage"			
"	0.140 0.04	7 0.047	0.147 0	c.m/sec"	
"	Catchment 202	Pervious	Impervious	Total Area	
"	Surface Area	0.024	0.255	0.279	hectare"
"	Time of concentration	7.860	1.438	1.865	minutes"
"	Time to Centroid	93.649	85.372	85.923	minutes"
"	Rainfall depth	93.224	93.224	93.224	mm"
"	Rainfall volume	22.11	237.99	260.10	c.m"
"	Rainfall losses	23.813	2.854	4.636	mm"
"	Runoff depth	69.411	90.370	88.588	mm"
"	Runoff volume	16.46	230.70	247.16	c.m"
"	Runoff coefficient	0.745	0.969	0.950	"
"	Maximum flow	0.012	0.134	0.140	c.m/sec"
"	40 HYDROGRAPH Add Runoff	"			
"	4 Add Runoff "				
"	0.140 0.16	0.047	0.147"		
"	54 POND DESIGN"				
"	0.160 Current peak flow	c.m/sec"			
"	0.057 Target outflow c	.m/sec"			
"	447.8 Hydrograph volume	c.m"			
"	21. Number of stages"				
"	420.140 Minimum water level	metre"			
"	420.490 Maximum water level	metre"			
"	420.140 Starting water leve	el metre"			
"	0 Keep Design Data: 1	= True; 0 =	= False"		
"	Level Discharge	Volume"			
"	420.140 0.000	0.000"			
"	420.158 0.04431	0.00803"			
"	420.175 0.04458	0.05914"			
"	420.193 0.04485	0.2056"			
"	420.210 0.04511	0.4737"			
"	420.228 0.04538	0.9415"			
"	420.245 0.04564	1.599"			
"	420.263 0.04591	2.572"			
"	420.280 0.04616	3.792"			
"	420.298 0.04643	5.452"			
"	420 315 0 04668	7.408"			
"	420 332 0 04693	9 783"			
"	420 350 0 04719	12 801"			
"	420 367 0 04744	16 168"			
"	420 385 0 04770	20 329"			
"	420.303 0.04770 420 402 0 04704	20.329 24 861"			
"	420.402 0.04794 420 420 0.04820	30 347"			
"	120.120 0.04020 120 130 0.01015	36 575"			
	720.730 0.04043	50.575			

.. 420.455 0.06061 43.198" " 420.473 0.1347 51.038" ... 420.490 0.2914 59.260" ... 1. WEIRS" ... Crest Weir Crest Left Right" ... elevation coefficie breadth sideslope sideslope" ... 420.440 0.900 0.000 380.000 380.000" ... ORIFICES" 1. " Orifice Orifice Orifice Number of" ... invert coefficie diameter orifices" ... 1.000" 418.640 0.630 0.1300 ... LAYERS" 1. ... Bottom Aspect Bottom Top Average" ... ratio elevation elevation sideslope" area ... 0.000 3.516 420.140 420.490 32.200" " 0.136 c.m/sec" Peak outflow ... Maximum level 420.473 metre" ... Maximum storage 51.111 c.m" ... Centroidal lag 1.732 hours" " 0.140 0.160 0.136 0.147 c.m/sec" HYDROGRAPH Next link " " 40 ... 5 Next link " " 0.140 0.136 0.136 0.147" ... 33 CATCHMENT 203" Triangular SCS" 1 ... 1 Equal length" ... Horton equation" 2 ... 203 C203 - SOUTH PARKING LOT AND REAR OF SOUTH SEMIS" .. 60.500 % Impervious" ... 0.321 Total Area" " 26.000 Flow length" ... 2.000 Overland Slope" ... 0.127 Pervious Area" ... 26.000 Pervious length" ... 2.000 Pervious slope" " 0.194 Impervious Area" ... 26.000 Impervious length" ... 2.000 Impervious slope" ... 0.250 Pervious Manning 'n'" ... 25.000 Pervious Max.infiltration" .. 5.000 Pervious Min.infiltration" ... 0.250 Pervious Lag constant (hours)" " 5.000 Pervious Depression storage" ... 0.015 Impervious Manning 'n'" ... 0.000 Impervious Max.infiltration" ... 0.000 Impervious Min.infiltration" ... 0.050 Impervious Lag constant (hours)" .. 1.500 Impervious Depression storage" ... 0.142 0.136 0.136 0.147 c.m/sec" " Catchment 203 Pervious Impervious Total Area " 0.127 0.194 " Surface Area 0.321 hectare" ... Time of concentration 8.247 1.508 3.760 minutes" 85.443 ... Time to Centroid 94.109 88.339 minutes" ... 93.224 Rainfall depth 93.224 93.224 mm" ... 181.05 Rainfall volume 118.20 299.25 c.m" ... Rainfall losses 23.710 2.790 mm" 11.054 ... mm" Runoff depth 69.515 90.434 82.171 " Runoff volume 88.14 175.63 263.77 C.m" ... Runoff coefficient 0.746 0.970 0.881 ... ... 0.142 c.m/sec" Maximum flow 0.061 0.102 **"** 40 HYDROGRAPH Add Runoff " ... 4 Add Runoff " ... 0.278 0.136 0.147" 0.142

"	54	PC	ND DESIGN	1"			
"		0.278	Current	peak flow	c.m/sec	c"	
"		0.079	Target o	outflow	c.m/sec"		
"		708.7	Hydrogra	aph volume	c.m"		
"		23.	Number o	of stages"			
"		419.960	Minimum	water leve	el metre	e"	
"		420.360	Maximum	water leve	el metre	e"	
"		419.960	Starting	g water lev	vel meti	re"	
"		0	Keep Des	sign Data:	1 = True;	0 = False'	,
"			Level	Discharge	Volume'	•	
"			419.960	0.000	0.000'	•	
"			419.979	0.07337	0.01123	•	
"			419.998	0.07387	0.08958'	•	
"			420.017	0.07436	0.3026'	•	
"			420.036	0.07485	0.7175'	•	
"			420.055	0.07533	1.400'	•	
"			420.074	0.07581	2.421	•	
"			420.093	0.07629	3.842'	•	
"			420.112	0.07676	5.737'	•	
"			420.131	0.07724	8.170'	•	
"			420.151	0.07773	11.383'	•	
"			420.169	0.07817	14.914'	•	
"			420.189	0.07866	19.616'	•	
"			420.208	0.07912	24.918'	•	
"			420.227	0.07958	31.089'	•	
"			420.246	0.08003	38.214'	•	
"			420.265	0.08194	46.353'	T	
"			420.284	0.1106	55.556	T	
"			420.303	0.1875	65.921	•	
"			420.322	0.3256	70.033'	•	
"			420.341	0.5364	70.033	•	
"			420.360	0.8288	70.033	•	
"			420.379	1.212	70.033	•	
"		1.	WEIRS"				
"			Crest	Weir	Crest	Left.	Right"
"		e	levation	coefficie	breadth	sideslope	sideslope"
"			420.260	0,900	2,000	180,000	180.000"
"		1.	ORIFICES	3"			
"			Orifice	Orifice	Orifice	Number of	,
"			invert	coefficie	diameter	orifices'	,
"			418.460	0.630	0.1680	1.000'	,
"		1.	LAYERS"	0.000	0.1000	1.000	
"			Bottom	Aspect	Bottom	тор	Average"
"			area	ratio	elevation	elevation	sideslope"
"			0.000	1.890	419.960	420.310	35.000"
"		Pe	ak outflo	)W	(	).232 C.	m/sec"
"		Ma	ximum lev	zel	42(	).314 me	etre"
"		Ma	ximum sto	prage	68	3.216 C.	m"
"		Ce	ntroidal	lag		1.750 hoi	irs"
"			0.142	0.278	0.232	0.147	c.m/sec"
	40	НУ	DROGRAPH	Next link	"	0.11/	0.111/000
"		.5	Next. lir	nk "			
"			0.1	42 0.3	232 0.2	232 0.1	47"
"	33	CA	TCHMENT	301"	- 0.2	0.1	·
"		1	Triangul	ar SCS"			
"		1	Equal le	ength"			
"		2	Horton e	auation"			
"		301	C301 - T	JNCONTROLL	ED PORTION		
"		46,400	% Imperv	vious"			
"		0.144	Total Ar	rea"			
"		6.000	Flow ler	nat.h"			
"		10.000	Overland	d Slope"			
"		0.077	Pervious	area"			

"	6.000	Pervious length"				
"	10.000	Pervious slope"				
"	0.067	Impervious Area"				
"	6.000	Impervious length"				
"	10.000	Impervious slope"				
"	0.250	Pervious Manning 'n	1 11			
"	25.000	Pervious Max.infilt	ration"			
"	5.000	Pervious Min.infilt	ration"			
"	0.250	Pervious Lag consta	nt (hours)"			
"	5.000	Pervious Depression	storage"			
"	0.015	Impervious Manning	'n'"			
"	0.000	Impervious Max.infi	ltration"			
"	0.000	Impervious Min.infi	ltration"			
"	0.050	Impervious Lag cons	tant (hours)	) "		
"	1.500	Impervious Depressi	on storage"			
"		0.071 0.23	2 0.232	0.147	c.m/sec	
"	C	atchment 301	Pervious	Impervious	Total A	Area "
"	S	Surface Area	0.077	0.067	0.144	hectare"
"	Τ	ime of concentration	2.111	0.386	1.245	minutes"
"	I	ime to Centroid	86.550	84.127	85.334	minutes"
"	F	ainfall depth	93.224	93.224	93.224	mm"
"	F	ainfall volume	71.95	62.29	134.24	c.m"
"	F	ainfall losses	24.458	13.192	19.231	mm"
"	F	unoff depth	68.766	80.032	73.993	mm"
"	F	Runoff volume	53.08	53.47	106.55	c.m"
"	F	unoff coefficient	0.738	0.858	0.794	"
	M	laximum flow	0.038	0.033	0.071	c.m/sec"
"	40 H	IYDROGRAPH Add Runoff	"			
	4	Add Runoff "				
		0.071 0.26	5 0.232	0.147"		
	40 H	IYDROGRAPH Copy to Out	flow"			
	8	Copy to Outflow"		0 1 4 5 4		
		0.071 0.26	5 0.265	0.14/"		
	40 H	IYDROGRAPH Combine	34"			
	6	Combine "				
	34	Node #"				
		LEAVING SITE"	0.27	20 ~ ~ ~ /~	!!	
	I.	laximum liow	1667 61	39 C.m/S	ec.	
	h	lydrograph volume	1557.53	32 C.M"		
	4.0 1	U.U/I U.20	5 U.205	0.339		
	40 n 7	Confluence "	e 54			
	3 V /	Nodo #"				
	54	NOUE #				
	N	LEAVING SILE	0.31	20 a m/a	o.o."	
	I	laximum liow	1557 53	29 C.III/S	ec	
	H		1001.00 1001.00			
	20 C		9 0.20J	0.000		
	ວບ ວ ຈ	Runoff Totale on TV	יב דיד <b>יי</b>			
	т	Intal Catchmont area	± ±	1	864	hectaro"
	ц	otal Impervious area		1	222	hectare"
	т Т	otal % impervious		65	.556"	neccure
"	19 F	XIT"		55		
	-					