

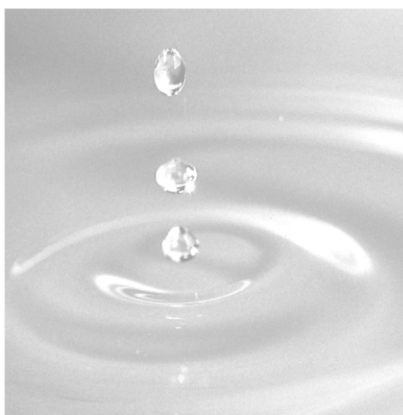
Functional Servicing and Stormwater Management Design Report – 191 Wellington Road 7 and 290 South Street

Township of Centre Wellington (Elora), Ontario

Submitted by:

GEI Consultants Canada Limited
330 Trillium Drive, Unit D
Kitchener, ON N2E 3J2
519.748.1440

October 29, 2025
Project No. 2404979



Patrick Grier, P.Eng.
Senior Project Engineer

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Certification

PREPARED BY:

GEI Consultants Canada Limited



Patrick Grier, P.Eng.
Senior Project Engineer

1. Introduction

This report has been prepared to document the servicing and stormwater management system design for the proposed development of 191 Wellington Road 7 and 290 South Street in the Township of Centre Wellington (Elora). The Owner is required to have a Professional Engineer design a stormwater management system and have the said Engineer supervise and certify that the stormwater management system was installed in accordance with the approvals given under Section 41 of the Planning Act.

The legal survey was completed by Black, Shoemaker, Robinson & Donaldson Limited (received December 6th, 2016). The topographical survey was completed by GM BluePlan Engineering Limited (dated December 6th, 2016). The existing and proposed site details are provided on the drawings prepared by GEI Consultants Canada Ltd.

The site is bound by South Street to the east, existing residential lands to the south and north, and Wellington Road 7 to the west. At this time, the intent of the Owner is to construct a 25-unit condominium development on the 0.74 ha site, along with the associated driving, parking and landscaped areas.

2. Site Grading

As illustrated by the topographic survey, the site generally slopes from the northeast to the southwest towards the roadside ditch along Wellington Road 7. The centerline road elevation on South Street along the frontage of the property ranges from approximately 395.0m to 392.0m. The edge of pavement elevation on Wellington Road 7 along the frontage of the property ranges from approximately 394.0m to 390.0m.

The grade and elevations of the site are controlled by the elevation of the existing road elevations of Wellington Road 7 and South Street. The site will be graded to match the existing elevations along the property limits.

3. Site Services

3.1. Water Supply

Water supply for majority of the site is proposed to be provided via the extension of a 150mm diameter water service lateral from the existing 150mm watermain on the South Street right-of-way. Water supply for Block 1 will be supplied via the extension of four (4) 25mm diameter water service laterals from the existing 150mm watermain on the South Street right-of-way.

Fire water supply will be provided by the existing municipal fire hydrants at the intersections of Wellington Road 7 and David Street West and David Street West and South Street.

Table 3-1. Anticipated Water Demands from Site

	Anticipated Water Demand
Average Water Demand (25 Units @ 2.4 ppl/unit @ 0.004 L/s/cap)	0.24 L/s
Maximum Day (Peak Factor of 2)	0.48 L/s

We estimate the fire flow demand for the largest townhouse block of the proposed development with noncombustible construction is 133 L/s, per the FUS approach. A copy of the fire flow demands have been included in Appendix A.

3.2. Sanitary Service

Sanitary service for the site will be provided by a 200mm diameter sanitary sewer extended from the existing 200mm diameter sanitary sewer on the South Street right-of-way. Sanitary services for Block 1 will be provided by the extension of four (4) 100mm diameter sanitary service laterals from the existing 200mm diameter sanitary sewer on South Street. The capacity of the existing 200mm diameter sanitary sewer on South Street is approximately 0.077 m³/s, based on a grade of 4.77%.

The following table summarizes the flows discharging from the site to the municipal sanitary sewer. The sanitary sewer design sheet is provided in Appendix B.

Table 3-2. Anticipated Sanitary Design Flows to South Street

	Anticipated Sanitary Design Flow
Peak Sanitary Sewer Flow (25 Units @ 2.4 ppl/unit @ 345.6 L/cap/d) Peak factor = 4	0.00127 m ³ /s
Extraneous Flow (0.75 ha site @ 0.15 L/s/ha)	0.00011 m ³ /s
Total Sanitary Flow	0.00139 m ³ /s
Existing 200mm Diameter Sanitary Sewer Capacity	0.077 m ³ /s

The peak sanitary sewer flow from the development to South Street represents approximately 2% of the existing sanitary sewer capacity. Therefore, in our opinion, the existing 200mm sanitary sewer on South Street has sufficient capacity to convey the anticipated design flows from the proposed development.

3.3. Storm Service

On-site storm sewers have been designed to convey the 100-year design storm event to the proposed detention tank. Storm services for the majority of the site will be provided via 300mm to 525mm diameter storm sewers discharging to the roadside ditch along the Wellington Road 7 right-of-way. A portion of the site will be serviced via a 250mm storm sewer discharging to the existing 250mm storm sewer on South Street. The storm sewer design sheet can be found in Appendix B.

4. Stormwater Management Design

The Fergus Shand Dam rainfall parameters presented in the Stormwater Management Master Plan for Centre Wellington (Elora, Salem and Fergus) (WSP Group, dated April 9, 2024) were used to generate the mass rainfall data for a 4-hour duration rainfall event for the full range of design storms (2 to 100-year). The Chicago storm parameters and the total depth of rainfall for each storm are as follows:

Table 4-1. Fergus Shand Dam – Chicago Storm Parameters

	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year
A	414.876	544.711	627.308	746.059	820.361	901.088
B	0.0268	0.0206	0.0136	0.0851	0.0100	0.0426
C	0.682	0.686	0.687	0.692	0.691	0.692
r	0.375	0.375	0.375	0.375	0.375	0.375
Duration (min.)	240	240	240	240	240	240
Total Depth (mm)	39.504	50.743	58.119	67.239	74.358	81.221

The Horton infiltration method was used in the MIDUSS model. The parameters used in MIDUSS are as follows:

Table 4-2. MIDUSS Horton Parameters

	Impervious Areas	Pervious Areas
Maximum Infiltration (mm/hr)	0.0	75.0
Minimum Infiltration (mm/hr)	0.0	12.5
Lag Constant (hr)	0.05	0.25
Depression Storage (mm)	1.5	5.0

The hydrologic model MIDUSS was used to create runoff hydrographs and to route the flows through the storage structures.

4.1. Stormwater Management Criteria

The following are the criteria used to design the stormwater management system for the proposed development:

1. The post-development peak runoff generated from the site is to be attenuated to the existing condition levels plus an additional 20% peak flow reduction, for the full range of design storm events up to and including the 100-year design storm.
2. Enhanced water quality treatment (80% TSS total suspended solids removal) is to be provided prior to the discharge of runoff from the site.
3. Major storm flows are to be routed overland to an appropriate outlet.

4.2. Existing Condition Drainage Areas

For the existing condition analysis, an analysis of two (2) drainage catchments was completed. The existing condition drainage areas are shown in Figure No. 1.

Catchment 10 (0.74-hectares, 2% impervious) represents the existing site, consisting of existing concrete foundations and landscaped areas. Under existing conditions, runoff generated from Catchment 10 sheetflows overland, uncontrolled, to the roadside ditch within the Wellington Road 7 right-of-way.

Catchment 20 (1.32-hectares, 20% impervious) represents the external drainage area northwest of the site, consisting of residential land use area with rooftops, driveways, and landscaped areas. Under existing conditions, runoff generated from Catchment 20 sheetflows overland, uncontrolled to Catchment 10 and eventually drains to the roadside ditch along Wellington Road 7 right-of-way.

The following table summarizes the existing condition flow rates.

Table 4-3. Existing Condition Peak Flow Rates

	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year
Catchment 10 (uncontrolled)	0.017 m ³ /s	0.064 m ³ /s	0.087 m ³ /s	0.0123 m ³ /s	0.158 m ³ /s	0.196 m ³ /s
Catchment 20 (uncontrolled)	0.062 m ³ /s	0.088 m ³ /s	0.105 m ³ /s	0.0128 m ³ /s	0.146 m ³ /s	0.165 m ³ /s
Total	0.075 m³/s	0.142 m³/s	0.186 m³/s	0.250 m³/s	0.304 m³/s	0.361 m³/s

4.3. Allowable Release Rates

The post-development flows are to be attenuated to the existing condition levels with an additional 20% water quantity control, for the full range of design storm events up to and including the 100-year design storm. Therefore, the allowable release rate under post-development conditions are as follows:

Table 4-4. Allowable Release Rates

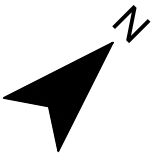
	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year
Total Flows from Site	0.060 m ³ /s	0.114 m ³ /s	0.149 m ³ /s	0.200 m ³ /s	0.243 m ³ /s	0.289 m ³ /s

4.4. Post-Development Condition Drainage Areas

For the post-development condition, an analysis of four (4) drainage catchments was completed. The post-development drainage catchments are shown on Figure No. 2.

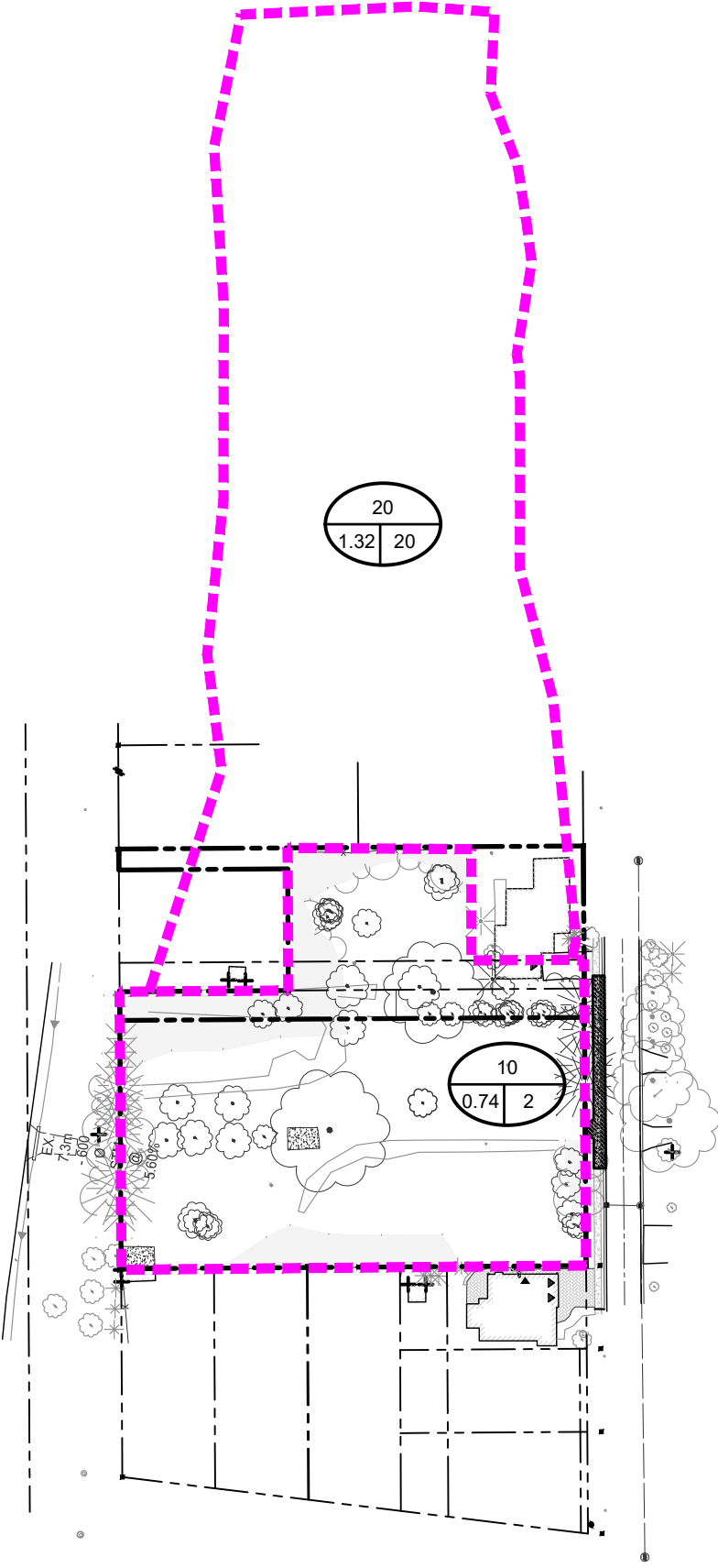
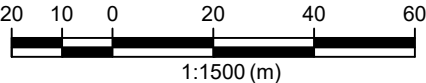
Catchment 100 (0.65-hectares, 65% impervious) represents the majority of the proposed development. Under post-development conditions, runoff generated from Catchment 100 will be captured and conveyed to on-site storm sewers to the proposed underground stormwater tank, ultimately discharging to the roadside ditch along the Wellington Road 7 right-of-way.

191 WELLINGTON ROAD 7
CENTRE WELLINGTON (ELORA)



LEGEND

- EX. STORM SEWER
- DRAINAGE AREA BOUNDARY
- CATCHMENT NUMBER
- % IMPERVIOUS
- CATCHMENT AREA IN HECTARES



PRE DEVELOPMENT
STORMWATER
MANAGEMENT FIGURE

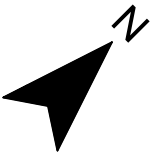
Figure No. 1



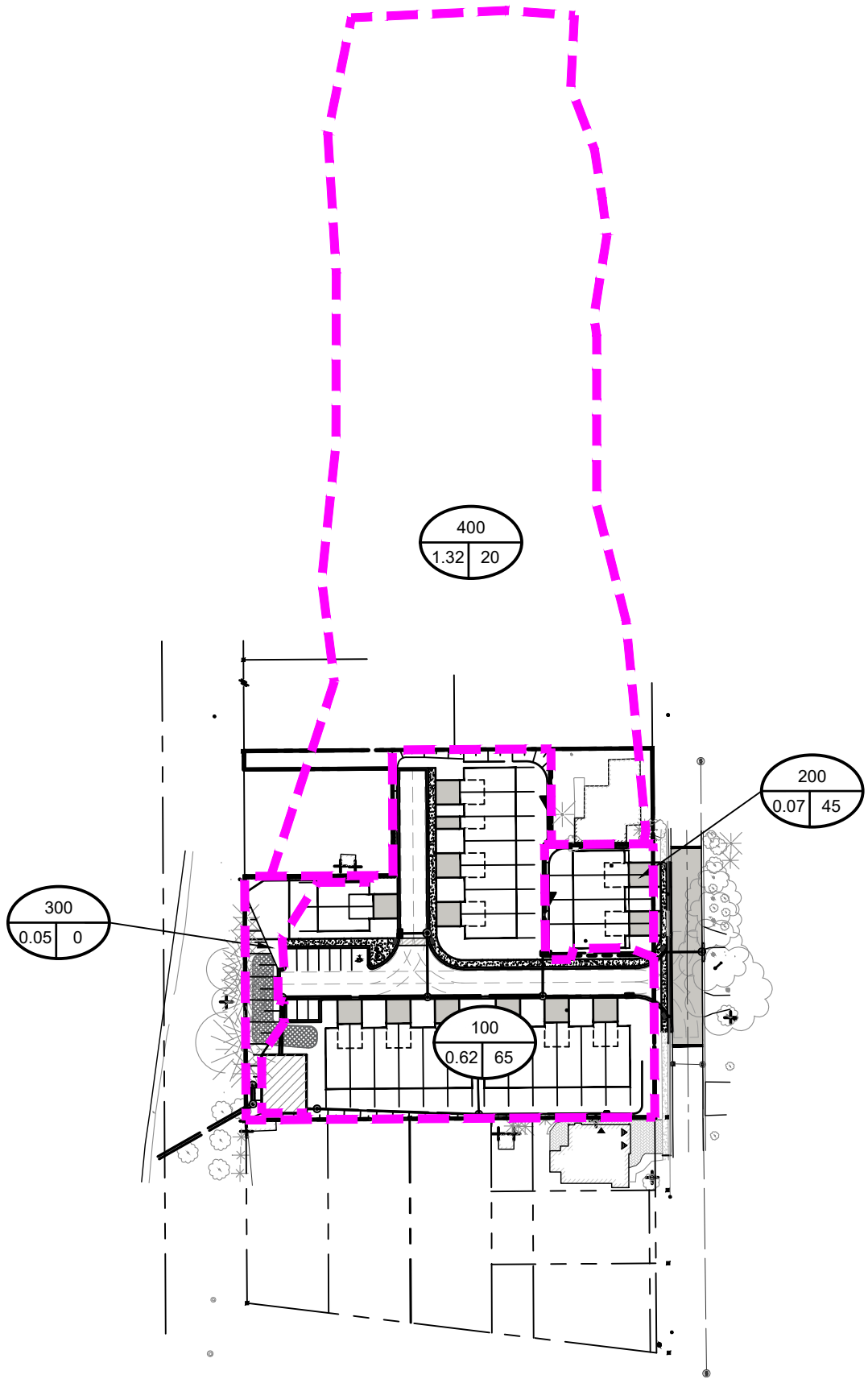
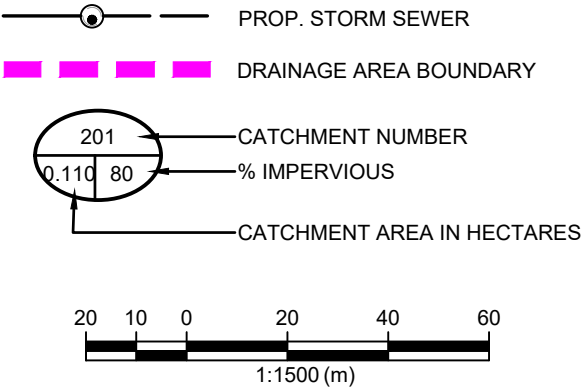
נוהל דיון פומבי: 191 WELLINGTON ROAD 7, CENTRE WELLINGTON (ELORA) | תאריך: 15.08.2025 | מסמך: 416149

FILE:B:\Working\James Keating Const\2404979 - 416149 Ross Property - 191 South Street, Elora\design Phase\Drawings\416149 - Design.dwg LAYOUT:POST DEVELOPMENT SWM FIG
LAST SAVED BY:Elora3933, 10/29/2025 10:59:29 AM PLOTTED BY:Armstrong, Erin 10/29/2025 11:01:18 AM

191 WELLINGTON ROAD 7
CENTRE WELLINGTON (ELORA)



LEGEND



POST DEVELOPMENT
STORMWATER
MANAGEMENT FIGURE

Figure No. 2



The proposed underground stormwater tank (12.81m length x 11.05m width x 1.36m depth) is anticipated to provide 184.8 m³ of storage volume and will be an EZ-Storm system or approved equivalent. Minor storm (2-year design storm) flows will be controlled via the proposed 190mm orifice, while 5-year and major storm flows will be controlled via the proposed 375mm outlet pipe. Storm sewers have been sized such that the 100-year design storm event flows from Catchment 100 and 400 are conveyed to the proposed stormwater tank.

Catchment 200 (0.07-hectares, 45% impervious) represents a portion of the townhouse rooftops, rearyards, and landscaped area along the northeast limit of the site. Under post-development conditions, runoff generated from Catchment 200 will either sheetflow uncontrolled or be conveyed via the rearyard swale and storm sewers to the South Street right-of-way.

Catchment 300 (0.05-hectares, 0% impervious) represents the landscaped area along the southwest limits of the site. Under post-development conditions, runoff generated from Catchment 300 is proposed to sheetflow overland, uncontrolled, towards the roadside ditch within the Wellington Road 7 right-of-way.

Catchment 400 (1.32-hectares, 20% impervious) represents the external drainage area northwest of the site (Catchment 20 under existing conditions), consisting of residential land use area with rooftops, driveways, and landscaped areas. Under post-development conditions, runoff generated from Catchment 400 will continue to sheetflow overland to Catchment 300, where flows will be controlled by the proposed underground stormwater tank before eventually discharging to the roadside ditch along the Wellington Road 7 right of way.

Enhanced level quality control will be provided for Catchment 100 and Catchment 400 via the proposed oil/grit separator structure (Stormceptor EFO6 or approved equivalent). Sizing calculations for the oil/grit separator are presented in Appendix D.

4.4.1. Routing

The Hydrologic model MIDUSS was used to create the 2-100-year design storm runoff hydrographs and to route the hydrographs. A copy of the modelling results of the post-development analysis is appended.

The results of the routing analysis are as follows:

Table 4-5. Catchment 300 Underground Stormwater Tank Storage Stage/Storage/Discharge Capacities

	Available Capacity			Actual Capacity Used		
	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m
190mm Orifice	0.000	0.0	389.23			
Bottom of Tank	0.000	0.0	389.24	---	---	---
2-Year	---	---	---	0.057	88.8	389.89
375mm Outlet	0.064	108.7	390.04	---	---	---
5-Year	---	---	---	0.095	124.8	390.16
10-Year	---	---	---	0.129	139.1	390.26
25-Year	---	---	---	0.184	155.2	390.38
50-Year	---	---	---	0.220	167.0	390.47
100-Year	---	---	---	0.261	184.2	390.60
Top of Tank and T/G CBMH.10	0.264	184.8	390.60			
Overflow	0.617	185.5	391.10	---	---	---

A summary of the post-development peak flow from the site for all design storm events are provided in the table below.

Table 4-6. Summary of Post-Development Peak Flow Rates

	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year
Catchment 100 (controlled)	0.057 m ³ /s	0.095 m ³ /s	0.129 m ³ /s	0.183 m ³ /s	0.220 m ³ /s	0.261 m ³ /s
Catchment 200 (uncontrolled)	0.012 m ³ /s	0.019 m ³ /s	0.017 m ³ /s	0.022 m ³ /s	0.026 m ³ /s	0.041 m ³ /s
Catchment 300 (uncontrolled)	0.005 m ³ /s	0.011 m ³ /s	0.011 m ³ /s	0.012 m ³ /s	0.016 m ³ /s	0.025 m ³ /s
Total	0.060m³/s	0.098 m³/s	0.133 m³/s	0.189 m³/s	0.227 m³/s	0.276 m³/s

Table 4-7. Comparison of Allowable Release Rates and Post-Development Flow Rates

	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year
Allowable Release Rate	0.060 m ³ /s	0.114 m ³ /s	0.149 m ³ /s	0.200 m ³ /s	0.243 m ³ /s	0.289 m ³ /s
Post-Development Flow Rate	0.060m ³ /s	0.098 m ³ /s	0.133 m ³ /s	0.189 m ³ /s	0.227 m ³ /s	0.276 m ³ /s

Therefore, the post-development flow rates have been attenuated to equal or less than the allowable release rates for the 2-through 100-year design storm events.

5. Maintenance Plan

To ensure that the stormwater management system continues to function as designed and constructed, we recommend that the following inspections and maintenance activities be completed on an annual basis:

1. Inspect the water level in the underground stormwater tank. Has the system completely drained within 48 hours after a storm?
2. Is there any noticeable damage to the underground stormwater tank (i.e. outlet structures, overflow weirs)? If yes, complete any necessary repairs and/or installation of replacement structures.
3. Is there any noticeable damage to the asphalt and grassed swales (i.e. erosion, blockages)? If yes, complete any necessary repairs.
4. Is there any indication of a spill (i.e. frothy water, oily sheen on the water)? If yes, investigate, inform the appropriate agencies and complete the necessary clean-up and restoration.
5. Inspect the oil/grit structure and complete any necessary maintenance/repair activities as identified by the manufacturer.
6. Inspect all catchbasins and manholes. Remove and dispose of any accumulated sediment, trash/litter, debris (i.e. sediment, garbage, leaves, etc.).
7. Inspect all swales and overflow locations. Remove and dispose of any accumulated sediment, trash/litter, debris (i.e. sediment, garbage, leaves, etc.).

Please note that any structures identified during the annual inspection to be worn, missing or damaged are to be repaired or replaced within 48 hours.

6. Erosion and Sediment Control Plan

A silt fence is to be installed along the perimeter of the property. The silt fence serves to minimize the opportunity for sediment to leave the site.

Inspection and maintenance of all silt fencing will start after installation is complete. The silt fence will be inspected on a weekly basis during active construction or after a rainfall event of 13 mm or greater. Maintenance will be carried out, within 48 hours, on any part of the silt fence found to need repair.

Once construction and landscaping has been substantially completed, the silt fence will be removed, any accumulated sediment will be removed, and the landscaping will be completed.

After construction of the complete development, erosion will not occur, and sediment transport will be minimal.

7. Conclusions

In summary, the features of the design for the proposed development are as follows:

1. It is proposed that the development will be accessed by South Street and Wellington Road 7 and that the grading of the development is controlled by the existing elevations of South Street and Wellington Road 7 across the frontages of the site.
2. Water supply for majority of the site is proposed to be provided via the extension of a 150mm diameter water service lateral from the existing 150mm watermain on the South Street right-of-way. Water supply for Block 1 will be supplied via the extension of four (4) 25mm diameter water service laterals from the existing 150mm watermain on the South Street right-of-way.
3. Sanitary service for the proposed development will be provided by a 200mm diameter sanitary sewer extended from the existing 200mm diameter sanitary sewer on the South Street right-of-way. Sanitary services for Block 1 will be provided by the extension of four (4) 100mm diameter sanitary service laterals from the existing 200mm diameter sanitary sewer on South Street.
4. Storm service for majority of the development will be provided via a 525mm diameter storm sewer discharging to the roadside ditch along the Wellington Road 7 right-of-way. A portion of the development will be serviced via a 250mm diameter storm sewer discharging to the existing 250mm storm sewer on South Street.
5. Post-development flow rates have been attenuated to equal or less than the allowable release rates for the 2 through 100-year design storm events.
6. Quality control treatment for runoff generated from Catchment 100 and Catchment 400 will be provided by the oil/grit separator structure (Stormceptor EF06 or approved equivalent) prior to discharge from the site.
7. Prior to construction, a silt fence will be installed along the property boundary in all locations where runoff will discharge from the site to adjacent lands. This will minimize the transport of sediment off-site during the construction period.

Appendix A Fire Flow Demands Analysis

Project No: 2404979		FIRE UNDERWRITERS SURVEY CALCULATIONS										Date: 9/3/2025	
Designed By: SJ		191 Wellington Road 7 Township of Centre Wellington (Elora)											
Parameters from Water Supply for Public Fire Protection, Fire Underwriters Survey (2020)												Proximity to closest structure (m)	
												0 to 3	25%
Type of Construction				Fire Hazard				Sprinklers				3 to 10	20%
Wood	1.5			Non-Combustible		-0.25		No		0		10 to 20	15%
Ordinary	1.0			Limited Combustibility		-0.15		System		-30%		20 to 30	10%
Noncombustible	0.8			Combustible		0		Standard Water Supply		-40%		Greater than 30	0%
Fire-Resistive	0.6			Free Burning		0.15		Fully Supervised		-50%			
				Rapid Burning		0.25							

Building	Gross Floor Area (m2)	Construction Coefficient	NFF 220*C*(A)**0.5	Rounded NFF (to nearest 1000L/min)	Occupancy Factor	NFF adjusted for occupancy	Sprinkler Adjustment	Sprinkler Credit	Exposure Coefficient					Exposure Flow Debit	RFF	Rounded RFF (to nearest 1000L/min)	Required Fire Flow (L/s)
									N	S	E	W	Total (max 0.75)				
Townhouse Units	1110	0.8	5864	6000	-0.15	5100	0	0	0.15	0.15	0	0.2	0.5	2550	7650	8000	133

Appendix B Sewer Design Sheets

Chicago Storm Parameters

Intensity = A / (t + B) ^ C

5-Year

A = 544.7

B = 0.021

C = 0.686

100-Year

A = 901.1

B = 0.043

C = 0.692

STORM SEWER DESIGN

100-Year Design Storm

Township of Centre Wellington (Elora)

Catchment Area	From	To	Area (ha)	Runoff Coefficient	A x C	Cumulative A x C	TC (min.)	Intensity (mm/hr)	5-yr Flow (m³/s)	100-yr Flow (m³/s)	Proposed Sewer							100-yr HGL							
											Length (m)	Pipe Size (mm)	Type of Pipe	Grade %	Capacity (m³/s)	Full Flow Velocity (m/s)	Time of Flow (min.)	HGL Grade %	HGL Capacity (m³/s)	U/S T/G (m)	U/S HGL (m)	D/S T/G (m)	D/S HGL (m)		
--	External Drainage Area	DCBMH.7	0.75	0.34	0.25	0.25	20.00	113.19	0.049	0.080															
--	External Drainage Area	CB.9	0.75	0.34	0.25	0.25	20.00	113.19	0.049	0.080															
8	DCBMH.12	CBMH.4	0.07	0.75	0.05	0.05	10.00	182.59	0.016	0.027	21.7	300	0.013	1.00	0.097	1.37	0.26	0.07	0.027	393.62	391.6474	393.75	391.6322		
10	CB.9	CBMH.4	0.07	0.75	0.05	0.31	20.00	113.19	0.059	0.096	10.5	300	0.013	2.00	0.137	1.93	0.09	1.11	0.106	393.79	391.7488	393.75	391.6322		
3	CBMH.4	CBMH.3	0.08	0.75	0.06	0.42	10.26	179.34	0.128	0.208	28.1	450	0.013	0.50	0.202	1.27	0.37	0.37	0.180	393.75	391.6322	393.92	391.5282		
1	DCBMH.7	CBMH.3	0.12	0.75	0.09	0.34	20.00	113.19	0.066	0.108	15.7	300	0.013	2.00	0.137	1.93	0.14	1.15	0.108	394.23	391.7088	393.92	391.5282		
9	CBMH.3	DCBMH.1	0.01	0.75	0.01	0.77	10.63	175.02	0.230	0.374	35.8	450	0.013	1.00	0.285	1.79	0.33	1.717	0.388	393.92	391.5282	393.33	390.9136		
4	DCBMH.1	TANK	0.11	0.75	0.08	0.85	10.97	171.34	0.249	0.405	15.6	450	0.013	3.00	0.494	3.10	0.08	2.01	0.420	393.33	390.9136	390.6	390.6		
5	CB.15	CBMH.11	0.04	0.75	0.03	0.03	10.00	182.59	0.009	0.015	31.5	250	0.013	0.50	0.042	0.86	0.61	0.06	0.015	391.72	389.7747	391.4	389.7558		
6	CBMH.11	CBMH.10	0.05	0.75	0.04	0.07	10.61	175.26	0.020	0.032	39.9	250	0.013	0.50	0.042	0.86	0.78	0.26	0.032	391.4	389.7558	391	389.652		
7	CBMH.10	TANK	0.08	0.75	0.06	0.13	11.39	166.93	0.036	0.058	2.6	250	0.013	1.00	0.059	1.21	0.04	0.87	0.058	391	389.652	390.6	389.6294		
--	TANK	MH.14	0.00	0.75	0.00	0.31	11.42	166.57	0.089	0.254	2.2	375	0.013	0.50	0.124	1.12	0.03	1.94	0.254	390.6	389.6294	391.65	389.5867		
											2.2	200	0.013	0.50	0.023	0.74	0.05								
--	MH.14	OGS	0.00	0.75	0.00	0.31	11.47	166.08	0.089	0.254	4.7	525	0.013	0.50	0.304	1.40	0.06	0.323	0.254	391.65	389.5867	389.36	389.5716		
--	OGS	OUTLET	0.00	0.75	0.00	0.31	11.46	166.25	0.089	0.254	26.8	525	0.013	0.50	0.304	1.40	0.32	0.323	0.254	390.36	389.5716	389.49	389.485		
2	CB.16	EX.MH17	0.04	0.75	0.03	0.03	10.00	182.59	0.009	0.015	37.7	250	0.013	1.00	0.059	1.21	0.52								
	The Cumulative AxC from TANK to OUTLET has been adjusted to reflect the 5-year flows from the proposed underground stormwater tank.					Date: October 28, 2025							Project: 191 Wellington Road 7												
Designed By: SJ																									
Checked by: PG							File: 2404979																		

PROJECT:191 Wellington Road 7
Township of Centre Wellington (Elora)

DATE:October 27, 2025

DESIGNED BY:S.J.

SANITARY SEWER DESIGN

Township of Centre Wellington

Average Daily Flow

Residential:345.6 L/c/d

Q(i) = Cum. Area (ha) * Infiltration Rate / 1000

Infiltration Rate:0.15 L/s/ha

Peaking Factor : F = 1 + (14/(4+P^0.5))

P = Population/1000

Manning Equation: Full Cap.= (D/2/1000)^2*Pi*(D/4/1000)^0.667*(1/n)*(S/100)^0.5

D = Diameter (mm)

S = Slope (%)

n = 0.013 (PVC & Concrete), 01016 (Vitrified Clay)

n =0.013

Minimum Full Velocity =0.80 m/s

Catchment	From M.H.	To M.H.	RESIDENTIAL AREA AND POPULATION							Commercial		Industrial		Institutional		C+I+I	Infiltration	Total Flow	Pipe					
			Area	No. of Units	Population	Cumulative		Peak Factor	Peak Flow	Area	Cum. Area	Area	Cum. Area	Area	Cum. Area	Peak Flow			Distance	Diameter	Slope	Capacity (Full)	Velocity	
			(ha)			Area (ha)	Population		(m³/s)	(ha)	(ha)	(ha)	(ha)	(ha)	(ha)	(m³/s)			(m)	(mm)	(%)	(m³/s)	Full (m/s)	Actual (m/s)
2	MH.D	MH.B	0.19	4	10	0.19	10	4.000	0.00015								0.000029	0.00018	16.90	200	1.00	0.0328	1.044	0.240
1	MH.C	MH.B	0.26	9	22	0.26	22	4.000	0.00035								0.000039	0.00039	48.10	200	4.00	0.0656	2.088	0.480
3	MH.B	MH.A	0.23	8	19	0.68	51	4.000	0.00081								0.000102	0.00091	58.30	200	1.00	0.0328	1.044	0.418
--	MH.A	EX.MH.1	0.00	0	0	0.68	51	4.000	0.00081								0.000102	0.00091	10.50	200	1.00	0.0328	1.044	0.418
4	Townhouse Building	Existing 200mm San	0.04	2	5	0.00	5	4.000	0.00008								0.000000	0.00008	14.25	100	1.00	0.0052	0.662	0.152
5	Townhouse Building	Existing 200mm San	0.04	2	5	0.00	5	4.000	0.00008								0.000000	0.00008	14.25	100	1.00	0.0052	0.662	0.152

GEI File: 240979

Appendix C Stormwater Management Analysis

Pre-Development

```
"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        \\geiconsultants.com\data\Data_Storage\"
"          Working\JAMES KEATING CONSTR\2404979 - 416149 Ross Property - 191
South Street, Elora\Design Phase\Design Calcs\Modelling Files\2025-10-27"
"          Output filename:                    2404979 Pre_2yr R2.out"
"          Licensee name:                      "
"          Company                            "
"          Date & Time last used:              10/27/2025 at 8:13:49 AM"
" 31          TIME PARAMETERS"
"          5.000  Time Step"
"          240.000  Max. Storm length"
"          1500.000  Max. Hydrograph"
" 32          STORM Chicago storm"
"          1  Chicago storm"
"          414.876  Coefficient A"
"          0.027  Constant B"
"          0.682  Exponent C"
"          0.375  Fraction R"
"          240.000  Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          100.234  mm/hr"
"          Total depth          39.504  mm"
"          6  002hyd  Hydrograph extension used in this file"
" 33          CATCHMENT 10"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          10  Catchment 10"
"          2.000  % Impervious"
"          0.740  Total Area"
"          65.000  Flow length"
"          7.000  Overland Slope"
"          0.725  Pervious Area"
"          65.000  Pervious length"
"          7.000  Pervious slope"
"          0.015  Impervious Area"
"          65.000  Impervious length"
"          7.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.050  Impervious Lag constant (hours)"
```


"	1.500	Impervious Depression storage"				
"		0.017	0.000	0.000	0.000 c.m/sec"	
"		Catchment 10	Pervious	Impervious	Total Area	"
"		Surface Area	0.725	0.015	0.740	hectare"
"		Time of concentration	19.735	2.420	16.189	minutes"
"		Time to Centroid	109.957	115.029	110.995	minutes"
"		Rainfall depth	39.504	39.504	39.504	mm"
"		Rainfall volume	286.48	5.85	292.33	c.m"
"		Rainfall losses	36.520	1.857	35.827	mm"
"		Runoff depth	2.984	37.647	3.677	mm"
"		Runoff volume	21.64	5.57	27.21	c.m"
"		Runoff coefficient	0.076	0.953	0.093	"
"		Maximum flow	0.016	0.004	0.017	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.017	0.017	0.000	0.000"	
" 33		CATCHMENT 20"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	20	Catchment 20"				
"	20.000	% Impervious"				
"	1.320	Total Area"				
"	300.000	Flow length"				
"	2.500	Overland Slope"				
"	1.056	Pervious Area"				
"	300.000	Pervious length"				
"	2.500	Pervious slope"				
"	0.264	Impervious Area"				
"	300.000	Impervious length"				
"	2.500	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	12.500	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.062	0.017	0.000	0.000 c.m/sec"	
"		Catchment 20	Pervious	Impervious	Total Area	"
"		Surface Area	1.056	0.264	1.320	hectare"
"		Time of concentration	67.284	8.252	22.428	minutes"
"		Time to Centroid	144.894	124.297	129.243	minutes"
"		Rainfall depth	39.504	39.504	39.504	mm"
"		Rainfall volume	417.16	104.29	521.45	c.m"
"		Rainfall losses	36.510	1.614	29.531	mm"
"		Runoff depth	2.994	37.890	9.973	mm"

"	Runoff volume	31.61	100.03	131.64	c.m"
"	Runoff coefficient	0.076	0.959	0.252	"
"	Maximum flow	0.009	0.061	0.062	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.062	0.075	0.000	0.000"
" 38	START/RE-START TOTALS 20"				
"	3 Runoff Totals on EXIT"				
"	Total Catchment area			2.060	hectare"
"	Total Impervious area			0.279	hectare"
"	Total % impervious			13.534"	
" 19	EXIT"				

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        \\geiconsultants.com\data\Data_Storage\"
"          Working\JAMES KEATING CONSTR\2404979 - 416149 Ross Property - 191
South Street, Elora\Design Phase\Design Calcs\Modelling Files\2025-10-27"
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"          Licensee name:                      "
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" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          240.000 Max. Storm length"
"          1500.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          544.711 Coefficient A"
"          0.021  Constant B"
"          0.686  Exponent C"
"          0.375  Fraction R"
"          240.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          130.577  mm/hr"
"          Total depth                50.743  mm"
"          6  005hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 10"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          10  Catchment 10"
"          2.000  % Impervious"
"          0.740  Total Area"
"          65.000  Flow length"
"          7.000  Overland Slope"
"          0.725  Pervious Area"
"          65.000  Pervious length"
"          7.000  Pervious slope"
"          0.015  Impervious Area"
"          65.000  Impervious length"
"          7.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.050  Impervious Lag constant (hours)"

```

"	1.500	Impervious Depression storage"				
"		0.064	0.000	0.000	0.000 c.m/sec"	
"		Catchment 10	Pervious	Impervious	Total Area	"
"		Surface Area	0.725	0.015	0.740	hectare"
"		Time of concentration	13.229	2.177	12.102	minutes"
"		Time to Centroid	107.321	113.802	107.982	minutes"
"		Rainfall depth	50.743	50.743	50.743	mm"
"		Rainfall volume	367.99	7.51	375.50	c.m"
"		Rainfall losses	41.977	1.952	41.177	mm"
"		Runoff depth	8.765	48.791	9.566	mm"
"		Runoff volume	63.57	7.22	70.79	c.m"
"		Runoff coefficient	0.173	0.962	0.189	"
"		Maximum flow	0.063	0.005	0.064	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.064	0.064	0.000	0.000"	
" 33		CATCHMENT 20"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	20	Catchment 20"				
"	20.000	% Impervious"				
"	1.320	Total Area"				
"	300.000	Flow length"				
"	2.500	Overland Slope"				
"	1.056	Pervious Area"				
"	300.000	Pervious length"				
"	2.500	Pervious slope"				
"	0.264	Impervious Area"				
"	300.000	Impervious length"				
"	2.500	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	12.500	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.088	0.064	0.000	0.000 c.m/sec"	
"		Catchment 20	Pervious	Impervious	Total Area	"
"		Surface Area	1.056	0.264	1.320	hectare"
"		Time of concentration	45.103	7.424	23.210	minutes"
"		Time to Centroid	134.920	122.191	127.524	minutes"
"		Rainfall depth	50.743	50.743	50.743	mm"
"		Rainfall volume	535.84	133.96	669.81	c.m"
"		Rainfall losses	41.901	1.693	33.860	mm"
"		Runoff depth	8.842	49.050	16.883	mm"

"	Runoff volume	93.37	129.49	222.86	c.m"
"	Runoff coefficient	0.174	0.967	0.333	"
"	Maximum flow	0.035	0.079	0.088	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.088	0.142	0.000	0.000"
" 38	START/RE-START TOTALS 20"				
"	3 Runoff Totals on EXIT"				
"	Total Catchment area			2.060	hectare"
"	Total Impervious area			0.279	hectare"
"	Total % impervious			13.534"	
" 19	EXIT"				

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        \\geiconsultants.com\data\Data_Storage\"
"          Working\JAMES KEATING CONSTR\2404979 - 416149 Ross Property - 191
South Street, Elora\Design Phase\Design Calcs\Modelling Files\2025-10-27"
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"          Licensee name:                      "
"          Company                            "
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" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          240.000 Max. Storm length"
"          1500.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          627.308 Coefficient A"
"          0.014  Constant B"
"          0.687  Exponent C"
"          0.375  Fraction R"
"          240.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          150.154  mm/hr"
"          Total depth                58.119  mm"
"          6  010hyd Hydrograph extension used in this file"
" 33      CATCHMENT 10"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          10  Catchment 10"
"          2.000  % Impervious"
"          0.740  Total Area"
"          65.000  Flow length"
"          7.000  Overland Slope"
"          0.725  Pervious Area"
"          65.000  Pervious length"
"          7.000  Pervious slope"
"          0.015  Impervious Area"
"          65.000  Impervious length"
"          7.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.050  Impervious Lag constant (hours)"

```

"	1.500	Impervious Depression storage"				
"		0.087	0.000	0.000	0.000 c.m/sec"	
"		Catchment 10	Pervious	Impervious	Total Area	"
"		Surface Area	0.725	0.015	0.740	hectare"
"		Time of concentration	12.039	2.059	11.248	minutes"
"		Time to Centroid	108.635	113.227	108.999	minutes"
"		Rainfall depth	58.119	58.119	58.119	mm"
"		Rainfall volume	421.48	8.60	430.08	c.m"
"		Rainfall losses	44.837	2.100	43.982	mm"
"		Runoff depth	13.282	56.019	14.137	mm"
"		Runoff volume	96.32	8.29	104.61	c.m"
"		Runoff coefficient	0.229	0.964	0.243	"
"		Maximum flow	0.086	0.006	0.087	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.087	0.087	0.000	0.000"	
" 33		CATCHMENT 20"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	20	Catchment 20"				
"	20.000	% Impervious"				
"	1.320	Total Area"				
"	300.000	Flow length"				
"	2.500	Overland Slope"				
"	1.056	Pervious Area"				
"	300.000	Pervious length"				
"	2.500	Pervious slope"				
"	0.264	Impervious Area"				
"	300.000	Impervious length"				
"	2.500	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	12.500	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.105	0.087	0.000	0.000 c.m/sec"	
"		Catchment 20	Pervious	Impervious	Total Area	"
"		Surface Area	1.056	0.264	1.320	hectare"
"		Time of concentration	41.044	7.020	23.552	minutes"
"		Time to Centroid	138.709	121.308	129.763	minutes"
"		Rainfall depth	58.119	58.119	58.119	mm"
"		Rainfall volume	613.74	153.43	767.17	c.m"
"		Rainfall losses	44.827	1.862	36.234	mm"
"		Runoff depth	13.292	56.257	21.885	mm"

"	Runoff volume	140.36	148.52	288.88	c.m"
"	Runoff coefficient	0.229	0.968	0.377	"
"	Maximum flow	0.052	0.091	0.105	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.105	0.186	0.000	0.000"
" 38	START/RE-START TOTALS 20"				
"	3 Runoff Totals on EXIT"				
"	Total Catchment area			2.060	hectare"
"	Total Impervious area			0.279	hectare"
"	Total % impervious			13.534"	
" 19	EXIT"				


```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        \\geiconsultants.com\data\Data_Storage\"
"          Working\JAMES KEATING CONSTR\2404979 - 416149 Ross Property - 191
South Street, Elora\Design Phase\Design Calcs\Modelling Files\2025-10-27"
"          Output filename:                    2404979 Pre_25yr R2.out"
"          Licensee name:                      "
"          Company                            "
"          Date & Time last used:              10/27/2025 at 8:15:56 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          240.000 Max. Storm length"
"          1500.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          746.059 Coefficient A"
"          0.085  Constant B"
"          0.692  Exponent C"
"          0.375  Fraction R"
"          240.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          175.654  mm/hr"
"          Total depth                67.239  mm"
"          6  025hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 10"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          10  Catchment 10"
"          2.000  % Impervious"
"          0.740  Total Area"
"          65.000  Flow length"
"          7.000  Overland Slope"
"          0.725  Pervious Area"
"          65.000  Pervious length"
"          7.000  Pervious slope"
"          0.015  Impervious Area"
"          65.000  Impervious length"
"          7.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.050  Impervious Lag constant (hours)"

```

"	1.500	Impervious Depression storage"				
"		0.123	0.000	0.000	0.000 c.m/sec"	
"		Catchment 10	Pervious	Impervious	Total Area	"
"		Surface Area	0.725	0.015	0.740	hectare"
"		Time of concentration	11.082	1.934	10.500	minutes"
"		Time to Centroid	109.116	112.498	109.331	minutes"
"		Rainfall depth	67.239	67.239	67.239	mm"
"		Rainfall volume	487.62	9.95	497.57	c.m"
"		Rainfall losses	47.713	2.249	46.804	mm"
"		Runoff depth	19.526	64.989	20.435	mm"
"		Runoff volume	141.60	9.62	151.22	c.m"
"		Runoff coefficient	0.290	0.967	0.304	"
"		Maximum flow	0.119	0.007	0.123	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.123	0.123	0.000	0.000"	
" 33		CATCHMENT 20"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	20	Catchment 20"				
"	20.000	% Impervious"				
"	1.320	Total Area"				
"	300.000	Flow length"				
"	2.500	Overland Slope"				
"	1.056	Pervious Area"				
"	300.000	Pervious length"				
"	2.500	Pervious slope"				
"	0.264	Impervious Area"				
"	300.000	Impervious length"				
"	2.500	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	12.500	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.128	0.123	0.000	0.000 c.m/sec"	
"		Catchment 20	Pervious	Impervious	Total Area	"
"		Surface Area	1.056	0.264	1.320	hectare"
"		Time of concentration	37.783	6.593	23.596	minutes"
"		Time to Centroid	138.702	120.175	130.274	minutes"
"		Rainfall depth	67.239	67.239	67.239	mm"
"		Rainfall volume	710.04	177.51	887.55	c.m"
"		Rainfall losses	47.710	2.058	38.580	mm"
"		Runoff depth	19.529	65.181	28.659	mm"

"	Runoff volume	206.22	172.08	378.30	c.m"
"	Runoff coefficient	0.290	0.969	0.426	"
"	Maximum flow	0.072	0.105	0.128	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.128	0.250	0.000	0.000"
" 38	START/RE-START TOTALS 20"				
"	3 Runoff Totals on EXIT"				
"	Total Catchment area			2.060	hectare"
"	Total Impervious area			0.279	hectare"
"	Total % impervious			13.534"	
" 19	EXIT"				

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        \\geiconsultants.com\data\Data_Storage\"
"          Working\JAMES KEATING CONSTR\2404979 - 416149 Ross Property - 191
South Street, Elora\Design Phase\Design Calcs\Modelling Files\2025-10-27"
"          Output filename:                    2404979 Pre_50yr R2.out"
"          Licensee name:                      "
"          Company                            "
"          Date & Time last used:              10/27/2025 at 8:16:32 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          240.000 Max. Storm length"
"          1500.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          820.361 Coefficient A"
"          0.010  Constant B"
"          0.691  Exponent C"
"          0.375  Fraction R"
"          240.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          194.803  mm/hr"
"          Total depth                74.358  mm"
"          6  050hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 10"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          10  Catchment 10"
"          2.000  % Impervious"
"          0.740  Total Area"
"          65.000  Flow length"
"          7.000  Overland Slope"
"          0.725  Pervious Area"
"          65.000  Pervious length"
"          7.000  Pervious slope"
"          0.015  Impervious Area"
"          65.000  Impervious length"
"          7.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.050  Impervious Lag constant (hours)"

```

"	1.500	Impervious Depression storage"				
"		0.158	0.000	0.000	0.000 c.m/sec"	
"		Catchment 10	Pervious	Impervious	Total Area	"
"		Surface Area	0.725	0.015	0.740	hectare"
"		Time of concentration	10.522	1.855	10.031	minutes"
"		Time to Centroid	109.478	112.168	109.631	minutes"
"		Rainfall depth	74.358	74.358	74.358	mm"
"		Rainfall volume	539.24	11.00	550.25	c.m"
"		Rainfall losses	49.899	2.420	48.949	mm"
"		Runoff depth	24.459	71.938	25.408	mm"
"		Runoff volume	177.38	10.65	188.02	c.m"
"		Runoff coefficient	0.329	0.967	0.342	"
"		Maximum flow	0.154	0.007	0.158	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.158	0.158	0.000	0.000"	
" 33		CATCHMENT 20"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	20	Catchment 20"				
"	20.000	% Impervious"				
"	1.320	Total Area"				
"	300.000	Flow length"				
"	2.500	Overland Slope"				
"	1.056	Pervious Area"				
"	300.000	Pervious length"				
"	2.500	Pervious slope"				
"	0.264	Impervious Area"				
"	300.000	Impervious length"				
"	2.500	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	12.500	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.146	0.158	0.000	0.000 c.m/sec"	
"		Catchment 20	Pervious	Impervious	Total Area	"
"		Surface Area	1.056	0.264	1.320	hectare"
"		Time of concentration	35.872	6.326	23.352	minutes"
"		Time to Centroid	138.555	119.576	130.513	minutes"
"		Rainfall depth	74.358	74.358	74.358	mm"
"		Rainfall volume	785.22	196.30	981.52	c.m"
"		Rainfall losses	49.791	2.095	40.252	mm"
"		Runoff depth	24.567	72.262	34.106	mm"

"	Runoff volume	259.43	190.77	450.20	c.m"
"	Runoff coefficient	0.330	0.972	0.459	"
"	Maximum flow	0.091	0.116	0.146	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.146	0.304	0.000	0.000"
" 38	START/RE-START TOTALS 20"				
"	3 Runoff Totals on EXIT"				
"	Total Catchment area			2.060	hectare"
"	Total Impervious area			0.279	hectare"
"	Total % impervious			13.534"	
" 19	EXIT"				

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        \\geiconsultants.com\data\Data_Storage\"
"          Working\JAMES KEATING CONSTR\2404979 - 416149 Ross Property - 191
South Street, Elora\Design Phase\Design Calcs\Modelling Files\2025-10-27"
"          Output filename:                    2404979 Pre_100yr R2.out"
"          Licensee name:                      "
"          Company                            "
"          Date & Time last used:              10/27/2025 at 8:17:03 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          240.000 Max. Storm length"
"          1500.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          901.088 Coefficient A"
"          0.043  Constant B"
"          0.692  Exponent C"
"          0.375  Fraction R"
"          240.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                    212.921  mm/hr"
"          Total depth                        81.221  mm"
"          6  100hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 10"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          10  Catchment 10"
"          2.000  % Impervious"
"          0.740  Total Area"
"          65.000  Flow length"
"          7.000  Overland Slope"
"          0.725  Pervious Area"
"          65.000  Pervious length"
"          7.000  Pervious slope"
"          0.015  Impervious Area"
"          65.000  Impervious length"
"          7.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.050  Impervious Lag constant (hours)"

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"	1.500	Impervious Depression storage"				
"		0.196	0.000	0.000	0.000 c.m/sec"	
"		Catchment 10	Pervious	Impervious	Total Area	"
"		Surface Area	0.725	0.015	0.740	hectare"
"		Time of concentration	10.083	1.791	9.655	minutes"
"		Time to Centroid	109.715	111.857	109.826	minutes"
"		Rainfall depth	81.221	81.221	81.221	mm"
"		Rainfall volume	589.01	12.02	601.03	c.m"
"		Rainfall losses	51.740	2.613	50.757	mm"
"		Runoff depth	29.481	78.607	30.463	mm"
"		Runoff volume	213.80	11.63	225.43	c.m"
"		Runoff coefficient	0.363	0.968	0.375	"
"		Maximum flow	0.192	0.008	0.196	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.196	0.196	0.000	0.000"	
" 33		CATCHMENT 20"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	20	Catchment 20"				
"	20.000	% Impervious"				
"	1.320	Total Area"				
"	300.000	Flow length"				
"	2.500	Overland Slope"				
"	1.056	Pervious Area"				
"	300.000	Pervious length"				
"	2.500	Pervious slope"				
"	0.264	Impervious Area"				
"	300.000	Impervious length"				
"	2.500	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	12.500	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.165	0.196	0.000	0.000 c.m/sec"	
"		Catchment 20	Pervious	Impervious	Total Area	"
"		Surface Area	1.056	0.264	1.320	hectare"
"		Time of concentration	34.377	6.105	23.078	minutes"
"		Time to Centroid	138.076	119.053	130.473	minutes"
"		Rainfall depth	81.221	81.221	81.221	mm"
"		Rainfall volume	857.69	214.42	1072.11	c.m"
"		Rainfall losses	51.517	2.126	41.639	mm"
"		Runoff depth	29.703	79.095	39.582	mm"

"	Runoff volume	313.67	208.81	522.48	c.m"
"	Runoff coefficient	0.366	0.974	0.487	"
"	Maximum flow	0.112	0.126	0.165	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.165	0.361	0.000	0.000"
" 38	START/RE-START TOTALS 20"				
"	3 Runoff Totals on EXIT"				
"	Total Catchment area			2.060	hectare"
"	Total Impervious area			0.279	hectare"
"	Total % impervious			13.534"	
" 19	EXIT"				

191 South Street, Township of Centre Wellington (Elora)

Our File: 2404979

October 27, 2025

Catchment 100: Proposed Underground Stormwater Tank

ELEV	INC. DEPTH	SURFACE AREA	INCR. VOL	ACCUM STORAGE VOL	
(m)	(m)	(m ²)	(m ³)	(m ³)	
389.24	0.00	141.55	0.00	0.00	Bottom of Tank
389.44	0.20	141.55	27.18	27.18	
389.64	0.40	141.55	27.18	54.36	
389.84	0.60	141.55	27.18	81.53	
390.04	0.80	141.55	27.18	108.71	Outlet 2
390.24	1.00	141.55	27.18	135.89	
390.44	1.20	141.55	27.18	163.07	
390.60	1.36	141.55	21.74	184.81	Top of Tank
391.00	1.76	0.36	0.14	184.95	Weir & T/G CBMH.10
391.10	1.86	10.00	0.52	185.47	Overflow

TANK DIMENSIONS

Length = 12.81 m
 Width = 11.05 m
 Height = 1.36 m
 Volume = 184.8 m³

Outlet 1 CALCULATION

Q = 0.100 m³/s
 Cd = 0.6
 H = 1.76 m
 2g = 19.62
 A = 0.028 m²
 D = 0.190 m
 D/2 = 0.095 m
 Elev = 389.23 masl

Outlet 2 CALCULATION

Q = 0.274 m³/s
 Cd = 0.6
 H = 0.87 m
 2g = 19.62
 A = 0.110 m²
 D = 0.375 m
 D/2 = 0.1875 m
 Elev = 390.04 masl

OVERFLOW WEIR

Q = 0.243 cu m/s
 d1 = 0.400 m
 h = 0.500 m
 H = 0.100 m
 2g = 19.620
 L = 6.000 m

DISCHARGE

ELEV	STAGE	STORAGE	OUTLET 1	OUTLET 2	WEIR	TOTAL	
(m)	(m)	(m ³)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	
389.24	0.00	0.00	0.000	0.000	0.000	0.000	Bottom of Tank
389.44	0.20	27.18	0.026	0.000	0.000	0.026	
389.64	0.40	54.36	0.042	0.000	0.000	0.042	
389.84	0.60	81.53	0.054	0.000	0.000	0.054	
390.04	0.80	108.71	0.064	0.000	0.000	0.064	Outlet 2
390.24	1.00	135.89	0.072	0.045	0.000	0.117	
390.44	1.20	163.07	0.080	0.135	0.000	0.215	
390.60	1.36	184.81	0.085	0.179	0.000	0.264	Top of Tank
391.00	1.76	184.95	0.098	0.258	0.000	0.356	Weir & T/G CBMH.10
391.10	1.86	185.47	0.100	0.274	0.243	0.617	Overflow

Post-Development

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        \\geiconsultants.com\data\Data_Storage\"
"          Working\JAMES KEATING CONSTR\2404979 - 416149 Ross Property - 191
South Street, Elora\Design Phase\Design Calcs\Modelling Files\2025-10-27"
"          Output filename:                      Post_2yr.out"
"          Licensee name:                      "
"          Company                              "
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" 31          TIME PARAMETERS"
"          3.000  Time Step"
"          240.000  Max. Storm length"
"          1500.000  Max. Hydrograph"
" 32          STORM Chicago storm"
"          1  Chicago storm"
"          414.876  Coefficient A"
"          0.027  Constant B"
"          0.682  Exponent C"
"          0.375  Fraction R"
"          240.000  Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          141.792  mm/hr"
"          Total depth          39.504  mm"
"          6  002hyd  Hydrograph extension used in this file"
" 33          CATCHMENT 200"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          200  Catchment 200"
"          45.000  % Impervious"
"          0.070  Total Area"
"          7.000  Flow length"
"          4.000  Overland Slope"
"          0.038  Pervious Area"
"          7.000  Pervious length"
"          4.000  Pervious slope"
"          0.032  Impervious Area"
"          7.000  Impervious length"
"          4.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.050  Impervious Lag constant (hours)"
```

"	1.500	Impervious Depression storage"				
"		0.012	0.000	0.000	0.000 c.m/sec"	
"		Catchment 200	Pervious	Impervious	Total Area	"
"		Surface Area	0.038	0.032	0.070	hectare"
"		Time of concentration	4.845	0.654	1.080	minutes"
"		Time to Centroid	97.618	111.269	109.882	minutes"
"		Rainfall depth	39.504	39.504	39.504	mm"
"		Rainfall volume	15.21	12.44	27.65	c.m"
"		Rainfall losses	36.097	2.675	21.057	mm"
"		Runoff depth	3.407	36.829	18.447	mm"
"		Runoff volume	1.31	11.60	12.91	c.m"
"		Runoff coefficient	0.086	0.932	0.467	"
"		Maximum flow	0.003	0.011	0.012	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.012	0.012	0.000	0.000"	
" 33		CATCHMENT 300"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	300	Catchment 300"				
"	0.000	% Impervious"				
"	0.050	Total Area"				
"	10.000	Flow length"				
"	10.000	Overland Slope"				
"	0.050	Pervious Area"				
"	10.000	Pervious length"				
"	10.000	Pervious slope"				
"	0.000	Impervious Area"				
"	10.000	Impervious length"				
"	10.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	12.500	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.005	0.012	0.000	0.000 c.m/sec"	
"		Catchment 300	Pervious	Impervious	Total Area	"
"		Surface Area	0.050	0.000	0.050	hectare"
"		Time of concentration	4.559	0.616	4.559	minutes"
"		Time to Centroid	97.341	111.133	97.341	minutes"
"		Rainfall depth	39.504	39.504	39.504	mm"
"		Rainfall volume	19.75	0.00	19.75	c.m"
"		Rainfall losses	36.115	2.810	36.115	mm"
"		Runoff depth	3.389	36.694	3.389	mm"

"	Runoff volume	1.69	0.00	1.69	c.m"
"	Runoff coefficient	0.086	0.000	0.086	"
"	Maximum flow	0.005	0.000	0.005	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.005	0.013	0.000	0.000"	
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.005	0.013	0.013	0.000"	
" 40	HYDROGRAPH Combine 1"				
"	6 Combine "				
"	1 Node #"				
"	Total Outflow from Site"				
"	Maximum flow	0.013		c.m/sec"	
"	Hydrograph volume	14.607		c.m"	
"	0.005	0.013	0.013	0.013"	
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"	0.005	0.000	0.013	0.013"	
" 33	CATCHMENT 400"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	400 Catchment 400"				
"	20.000 % Impervious"				
"	1.320 Total Area"				
"	300.000 Flow length"				
"	2.500 Overland Slope"				
"	1.056 Pervious Area"				
"	300.000 Pervious length"				
"	2.500 Pervious slope"				
"	0.264 Impervious Area"				
"	300.000 Impervious length"				
"	2.500 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	75.000 Pervious Max.infiltration"				
"	12.500 Pervious Min.infiltration"				
"	0.250 Pervious Lag constant (hours)"				
"	5.000 Pervious Depression storage"				
"	0.015 Impervious Manning 'n'"				
"	0.000 Impervious Max.infiltration"				
"	0.000 Impervious Min.infiltration"				
"	0.050 Impervious Lag constant (hours)"				
"	1.500 Impervious Depression storage"				
"	0.068	0.000	0.013	0.013 c.m/sec"	
"	Catchment 400	Pervious	Impervious	Total Area	"
"	Surface Area	1.056	0.264	1.320	hectare"
"	Time of concentration	53.182	7.183	19.340	minutes"
"	Time to Centroid	136.914	122.873	126.584	minutes"
"	Rainfall depth	39.504	39.504	39.504	mm"

"	Rainfall volume	417.16	104.29	521.45	c.m"
"	Rainfall losses	36.095	1.544	29.185	mm"
"	Runoff depth	3.409	37.960	10.319	mm"
"	Runoff volume	36.00	100.21	136.21	c.m"
"	Runoff coefficient	0.086	0.961	0.261	"
"	Maximum flow	0.013	0.067	0.068	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.068	0.068	0.013	0.013"
" 33	CATCHMENT 100"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	100 Catchment 100"				
"	65.000 % Impervious"				
"	0.620 Total Area"				
"	50.000 Flow length"				
"	3.000 Overland Slope"				
"	0.217 Pervious Area"				
"	50.000 Pervious length"				
"	3.000 Pervious slope"				
"	0.403 Impervious Area"				
"	50.000 Impervious length"				
"	3.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	75.000 Pervious Max.infiltration"				
"	12.500 Pervious Min.infiltration"				
"	0.250 Pervious Lag constant (hours)"				
"	5.000 Pervious Depression storage"				
"	0.015 Impervious Manning 'n'"				
"	0.000 Impervious Max.infiltration"				
"	0.000 Impervious Min.infiltration"				
"	0.050 Impervious Lag constant (hours)"				
"	1.500 Impervious Depression storage"				
"		0.135	0.068	0.013	0.013 c.m/sec"
"	Catchment 100	Pervious	Impervious	Total Area	"
"	Surface Area	0.217	0.403	0.620	hectare"
"	Time of concentration	17.184	2.321	3.013	minutes"
"	Time to Centroid	107.467	114.292	113.974	minutes"
"	Rainfall depth	39.504	39.504	39.504	mm"
"	Rainfall volume	85.72	159.20	244.92	c.m"
"	Rainfall losses	36.106	2.048	13.968	mm"
"	Runoff depth	3.398	37.456	25.536	mm"
"	Runoff volume	7.37	150.95	158.32	c.m"
"	Runoff coefficient	0.086	0.948	0.646	"
"	Maximum flow	0.007	0.134	0.135	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.135	0.175	0.013	0.013"
" 54	POND DESIGN"				

"	0.175	Current peak flow	c.m/sec"		
"	0.270	Target outflow	c.m/sec"		
"	294.5	Hydrograph volume	c.m"		
"	10.	Number of stages"			
"	392.080	Minimum water level	metre"		
"	393.950	Maximum water level	metre"		
"	392.080	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	389.240	0.000	0.000"		
"	389.440	0.02600	27.180"		
"	389.640	0.04200	54.360"		
"	389.840	0.05400	81.530"		
"	390.040	0.06400	108.710"		
"	390.240	0.1170	135.890"		
"	390.440	0.2150	163.070"		
"	390.600	0.2640	184.810"		
"	391.000	0.3560	184.950"		
"	391.100	0.6170	185.470"		
"		Peak outflow	0.057	c.m/sec"	
"		Maximum level	389.893	metre"	
"		Maximum storage	88.753	c.m"	
"		Centroidal lag	2.350	hours"	
"	0.135	0.175	0.057	0.013 c.m/sec"	
" 40		HYDROGRAPH Combine	1"		
"	6	Combine "			
"	1	Node #"			
"		Total Outflow from Site"			
"		Maximum flow	0.060	c.m/sec"	
"		Hydrograph volume	309.081	c.m"	
"		0.135	0.175	0.057	0.060"
" 38		START/RE-START TOTALS 100"			
"	3	Runoff Totals on EXIT"			
"		Total Catchment area	2.060	hectare"	
"		Total Impervious area	0.699	hectare"	
"		Total % impervious	33.908"		
" 19		EXIT"			

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        \\geiconsultants.com\data\Data_Storage\"
"          Working\JAMES KEATING CONSTR\2404979 - 416149 Ross Property - 191
South Street, Elora\Design Phase\Design Calcs\Modelling Files\2025-10-27"
"          Output filename:                      Post_5yr.out"
"          Licensee name:                      "
"          Company                              "
"          Date & Time last used:                10/29/2025 at 1:25:32 PM"
" 31          TIME PARAMETERS"
"          3.000  Time Step"
"          240.000  Max. Storm length"
"          1500.000  Max. Hydrograph"
" 32          STORM Chicago storm"
"          1  Chicago storm"
"          544.711  Coefficient A"
"          0.021  Constant B"
"          0.686  Exponent C"
"          0.375  Fraction R"
"          240.000  Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          185.154  mm/hr"
"          Total depth          50.743  mm"
"          6  005hyd  Hydrograph extension used in this file"
" 33          CATCHMENT 200"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          200  Catchment 200"
"          45.000  % Impervious"
"          0.070  Total Area"
"          7.000  Flow length"
"          4.000  Overland Slope"
"          0.038  Pervious Area"
"          7.000  Pervious length"
"          4.000  Pervious slope"
"          0.032  Impervious Area"
"          7.000  Impervious length"
"          4.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.050  Impervious Lag constant (hours)"

```


"	1.500	Impervious Depression storage"				
"		0.019	0.000	0.000	0.000 c.m/sec"	
"		Catchment 200	Pervious	Impervious	Total Area	"
"		Surface Area	0.038	0.032	0.070	hectare"
"		Time of concentration	3.455	0.588	1.122	minutes"
"		Time to Centroid	97.331	110.169	107.780	minutes"
"		Rainfall depth	50.743	50.743	50.743	mm"
"		Rainfall volume	19.54	15.98	35.52	c.m"
"		Rainfall losses	41.872	3.319	24.524	mm"
"		Runoff depth	8.871	47.423	26.219	mm"
"		Runoff volume	3.42	14.94	18.35	c.m"
"		Runoff coefficient	0.175	0.935	0.517	"
"		Maximum flow	0.009	0.015	0.019	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.019	0.019	0.000	0.000"	
" 33		CATCHMENT 300"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	300	Catchment 300"				
"	0.000	% Impervious"				
"	0.050	Total Area"				
"	10.000	Flow length"				
"	10.000	Overland Slope"				
"	0.050	Pervious Area"				
"	10.000	Pervious length"				
"	10.000	Pervious slope"				
"	0.000	Impervious Area"				
"	10.000	Impervious length"				
"	10.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	12.500	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.011	0.019	0.000	0.000 c.m/sec"	
"		Catchment 300	Pervious	Impervious	Total Area	"
"		Surface Area	0.050	0.000	0.050	hectare"
"		Time of concentration	3.251	0.553	3.251	minutes"
"		Time to Centroid	97.137	110.036	97.137	minutes"
"		Rainfall depth	50.743	50.743	50.743	mm"
"		Rainfall volume	25.37	0.00	25.37	c.m"
"		Rainfall losses	41.842	3.473	41.842	mm"
"		Runoff depth	8.900	47.270	8.900	mm"

"	Runoff volume	4.45	0.00	4.45	c.m"
"	Runoff coefficient	0.175	0.000	0.175	"
"	Maximum flow	0.011	0.000	0.011	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.011	0.025	0.000	0.000"	
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.011	0.025	0.025	0.000"	
" 40	HYDROGRAPH Combine 1"				
"	6 Combine "				
"	1 Node #"				
"	Total Outflow from Site"				
"	Maximum flow	0.025		c.m/sec"	
"	Hydrograph volume	22.804		c.m"	
"	0.011	0.025	0.025	0.025"	
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"	0.011	0.000	0.025	0.025"	
" 33	CATCHMENT 400"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	400 Catchment 400"				
"	20.000 % Impervious"				
"	1.320 Total Area"				
"	300.000 Flow length"				
"	2.500 Overland Slope"				
"	1.056 Pervious Area"				
"	300.000 Pervious length"				
"	2.500 Pervious slope"				
"	0.264 Impervious Area"				
"	300.000 Impervious length"				
"	2.500 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	75.000 Pervious Max.infiltration"				
"	12.500 Pervious Min.infiltration"				
"	0.250 Pervious Lag constant (hours)"				
"	5.000 Pervious Depression storage"				
"	0.015 Impervious Manning 'n'"				
"	0.000 Impervious Max.infiltration"				
"	0.000 Impervious Min.infiltration"				
"	0.050 Impervious Lag constant (hours)"				
"	1.500 Impervious Depression storage"				
"	0.100	0.000	0.025	0.025 c.m/sec"	
"	Catchment 400	Pervious	Impervious	Total Area	"
"	Surface Area	1.056	0.264	1.320	hectare"
"	Time of concentration	37.920	6.456	19.683	minutes"
"	Time to Centroid	130.245	120.797	124.769	minutes"
"	Rainfall depth	50.743	50.743	50.743	mm"

"	Rainfall volume	535.84	133.96	669.81	c.m"
"	Rainfall losses	41.834	1.605	33.788	mm"
"	Runoff depth	8.909	49.138	16.955	mm"
"	Runoff volume	94.08	129.72	223.81	c.m"
"	Runoff coefficient	0.176	0.968	0.334	"
"	Maximum flow	0.041	0.092	0.100	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.100	0.100	0.025	0.025"
" 33	CATCHMENT 100"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	100 Catchment 100"				
"	65.000 % Impervious"				
"	0.620 Total Area"				
"	50.000 Flow length"				
"	3.000 Overland Slope"				
"	0.217 Pervious Area"				
"	50.000 Pervious length"				
"	3.000 Pervious slope"				
"	0.403 Impervious Area"				
"	50.000 Impervious length"				
"	3.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	75.000 Pervious Max.infiltration"				
"	12.500 Pervious Min.infiltration"				
"	0.250 Pervious Lag constant (hours)"				
"	5.000 Pervious Depression storage"				
"	0.015 Impervious Manning 'n'"				
"	0.000 Impervious Max.infiltration"				
"	0.000 Impervious Min.infiltration"				
"	0.050 Impervious Lag constant (hours)"				
"	1.500 Impervious Depression storage"				
"		0.183	0.100	0.025	0.025 c.m/sec"
"	Catchment 100	Pervious	Impervious	Total Area	"
"	Surface Area	0.217	0.403	0.620	hectare"
"	Time of concentration	12.252	2.086	3.005	minutes"
"	Time to Centroid	105.715	113.074	112.409	minutes"
"	Rainfall depth	50.743	50.743	50.743	mm"
"	Rainfall volume	110.11	204.49	314.61	c.m"
"	Rainfall losses	41.833	2.450	16.234	mm"
"	Runoff depth	8.910	48.293	34.509	mm"
"	Runoff volume	19.34	194.62	213.96	c.m"
"	Runoff coefficient	0.176	0.952	0.680	"
"	Maximum flow	0.021	0.179	0.183	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.183	0.244	0.025	0.025"
" 54	POND DESIGN"				

"	0.244	Current peak flow	c.m/sec"		
"	0.270	Target outflow	c.m/sec"		
"	437.8	Hydrograph volume	c.m"		
"	10.	Number of stages"			
"	392.080	Minimum water level	metre"		
"	393.950	Maximum water level	metre"		
"	392.080	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	389.240	0.000	0.000"		
"	389.440	0.02600	27.180"		
"	389.640	0.04200	54.360"		
"	389.840	0.05400	81.530"		
"	390.040	0.06400	108.710"		
"	390.240	0.1170	135.890"		
"	390.440	0.2150	163.070"		
"	390.600	0.2640	184.810"		
"	391.000	0.3560	184.950"		
"	391.100	0.6170	185.470"		
"		Peak outflow	0.095	c.m/sec"	
"		Maximum level	390.158	metre"	
"		Maximum storage	124.760	c.m"	
"		Centroidal lag	2.354	hours"	
"	0.183	0.244	0.095	0.025 c.m/sec"	
" 40		HYDROGRAPH Combine	1"		
"	6	Combine "			
"	1	Node #"			
"		Total Outflow from Site"			
"		Maximum flow	0.098	c.m/sec"	
"		Hydrograph volume	460.710	c.m"	
"	0.183	0.244	0.095	0.098"	
" 38		START/RE-START TOTALS 100"			
"	3	Runoff Totals on EXIT"			
"		Total Catchment area	2.060	hectare"	
"		Total Impervious area	0.699	hectare"	
"		Total % impervious	33.908"		
" 19		EXIT"			

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        \\geiconsultants.com\data\Data_Storage\"
"          Working\JAMES KEATING CONSTR\2404979 - 416149 Ross Property - 191
South Street, Elora\Design Phase\Design Calcs\Modelling Files\2025-10-27"
"          Output filename:                    Post_10yr.out"
"          Licensee name:                      "
"          Company                            "
"          Date & Time last used:              10/29/2025 at 1:26:12 PM"
" 31          TIME PARAMETERS"
"          5.000  Time Step"
"          240.000  Max. Storm length"
"          1500.000  Max. Hydrograph"
" 32          STORM Chicago storm"
"          1  Chicago storm"
"          627.308  Coefficient A"
"          0.014  Constant B"
"          0.687  Exponent C"
"          0.375  Fraction R"
"          240.000  Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          150.154  mm/hr"
"          Total depth          58.119  mm"
"          6  010hyd  Hydrograph extension used in this file"
" 33          CATCHMENT 200"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          200  Catchment 200"
"          45.000  % Impervious"
"          0.070  Total Area"
"          7.000  Flow length"
"          4.000  Overland Slope"
"          0.038  Pervious Area"
"          7.000  Pervious length"
"          4.000  Pervious slope"
"          0.032  Impervious Area"
"          7.000  Impervious length"
"          4.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.050  Impervious Lag constant (hours)"

```

"	1.500	Impervious Depression storage"				
"		0.017	0.000	0.000	0.000 c.m/sec"	
"		Catchment 200	Pervious	Impervious	Total Area	"
"		Surface Area	0.038	0.032	0.070	hectare"
"		Time of concentration	3.739	0.640	1.340	minutes"
"		Time to Centroid	99.816	111.634	108.964	minutes"
"		Rainfall depth	58.119	58.119	58.119	mm"
"		Rainfall volume	22.38	18.31	40.68	c.m"
"		Rainfall losses	45.394	4.836	27.143	mm"
"		Runoff depth	12.725	53.283	30.976	mm"
"		Runoff volume	4.90	16.78	21.68	c.m"
"		Runoff coefficient	0.219	0.917	0.533	"
"		Maximum flow	0.007	0.011	0.017	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.017	0.017	0.000	0.000"	
" 33		CATCHMENT 300"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	300	Catchment 300"				
"	0.000	% Impervious"				
"	0.050	Total Area"				
"	10.000	Flow length"				
"	10.000	Overland Slope"				
"	0.050	Pervious Area"				
"	10.000	Pervious length"				
"	10.000	Pervious slope"				
"	0.000	Impervious Area"				
"	10.000	Impervious length"				
"	10.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	12.500	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.010	0.017	0.000	0.000 c.m/sec"	
"		Catchment 300	Pervious	Impervious	Total Area	"
"		Surface Area	0.050	0.000	0.050	hectare"
"		Time of concentration	3.518	0.602	3.518	minutes"
"		Time to Centroid	99.582	111.671	99.582	minutes"
"		Rainfall depth	58.119	58.119	58.119	mm"
"		Rainfall volume	29.06	0.00	29.06	c.m"
"		Rainfall losses	45.386	5.162	45.386	mm"
"		Runoff depth	12.733	52.957	12.733	mm"

"	Runoff volume	6.37	0.00	6.37	c.m"
"	Runoff coefficient	0.219	0.000	0.219	"
"	Maximum flow	0.010	0.000	0.010	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.010 0.026 0.000 0.000"				
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.010 0.026 0.026 0.000"				
" 40	HYDROGRAPH Combine 1"				
"	6 Combine "				
"	1 Node #"				
"	Total Outflow from Site"				
"	Maximum flow	0.026		c.m/sec"	
"	Hydrograph volume	28.050		c.m"	
"	0.010 0.026 0.026 0.026"				
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"	0.010 0.000 0.026 0.026"				
" 33	CATCHMENT 400"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	400 Catchment 400"				
"	20.000 % Impervious"				
"	1.320 Total Area"				
"	300.000 Flow length"				
"	2.500 Overland Slope"				
"	1.056 Pervious Area"				
"	300.000 Pervious length"				
"	2.500 Pervious slope"				
"	0.264 Impervious Area"				
"	300.000 Impervious length"				
"	2.500 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	75.000 Pervious Max.infiltration"				
"	12.500 Pervious Min.infiltration"				
"	0.250 Pervious Lag constant (hours)"				
"	5.000 Pervious Depression storage"				
"	0.015 Impervious Manning 'n'"				
"	0.000 Impervious Max.infiltration"				
"	0.000 Impervious Min.infiltration"				
"	0.050 Impervious Lag constant (hours)"				
"	1.500 Impervious Depression storage"				
"	0.105 0.000 0.026 0.026 c.m/sec"				
"	Catchment 400 Pervious Impervious Total Area "				
"	Surface Area 1.056 0.264 1.320 hectare"				
"	Time of concentration 41.044 7.020 23.552 minutes"				
"	Time to Centroid 138.709 121.308 129.763 minutes"				
"	Rainfall depth 58.119 58.119 58.119 mm"				

"	Rainfall volume	613.74	153.43	767.17	c.m"
"	Rainfall losses	44.827	1.862	36.234	mm"
"	Runoff depth	13.292	56.257	21.885	mm"
"	Runoff volume	140.36	148.52	288.88	c.m"
"	Runoff coefficient	0.229	0.968	0.377	"
"	Maximum flow	0.052	0.091	0.105	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.105	0.105	0.026	0.026"
" 33	CATCHMENT 100"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	100 Catchment 100"				
"	65.000 % Impervious"				
"	0.620 Total Area"				
"	50.000 Flow length"				
"	3.000 Overland Slope"				
"	0.217 Pervious Area"				
"	50.000 Pervious length"				
"	3.000 Pervious slope"				
"	0.403 Impervious Area"				
"	50.000 Impervious length"				
"	3.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	75.000 Pervious Max.infiltration"				
"	12.500 Pervious Min.infiltration"				
"	0.250 Pervious Lag constant (hours)"				
"	5.000 Pervious Depression storage"				
"	0.015 Impervious Manning 'n'"				
"	0.000 Impervious Max.infiltration"				
"	0.000 Impervious Min.infiltration"				
"	0.050 Impervious Lag constant (hours)"				
"	1.500 Impervious Depression storage"				
"		0.161	0.105	0.026	0.026 c.m/sec"
"	Catchment 100	Pervious	Impervious	Total Area	"
"	Surface Area	0.217	0.403	0.620	hectare"
"	Time of concentration	13.262	2.268	3.505	minutes"
"	Time to Centroid	109.871	113.591	113.172	minutes"
"	Rainfall depth	58.119	58.119	58.119	mm"
"	Rainfall volume	126.12	234.22	360.34	c.m"
"	Rainfall losses	44.910	1.986	17.009	mm"
"	Runoff depth	13.209	56.134	41.110	mm"
"	Runoff volume	28.66	226.22	254.88	c.m"
"	Runoff coefficient	0.227	0.966	0.707	"
"	Maximum flow	0.025	0.154	0.161	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.161	0.236	0.026	0.026"
" 54	POND DESIGN"				

"	0.236	Current peak flow	c.m/sec"		
"	0.270	Target outflow	c.m/sec"		
"	543.8	Hydrograph volume	c.m"		
"	10.	Number of stages"			
"	392.080	Minimum water level	metre"		
"	393.950	Maximum water level	metre"		
"	392.080	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	389.240	0.000	0.000"		
"	389.440	0.02600	27.180"		
"	389.640	0.04200	54.360"		
"	389.840	0.05400	81.530"		
"	390.040	0.06400	108.710"		
"	390.240	0.1170	135.890"		
"	390.440	0.2150	163.070"		
"	390.600	0.2640	184.810"		
"	391.000	0.3560	184.950"		
"	391.100	0.6170	185.470"		
"		Peak outflow	0.129	c.m/sec"	
"		Maximum level	390.264	metre"	
"		Maximum storage	139.101	c.m"	
"		Centroidal lag	2.394	hours"	
"	0.161	0.236	0.129	0.026 c.m/sec"	
" 40		HYDROGRAPH Combine	1"		
"	6	Combine "			
"	1	Node #"			
"		Total Outflow from Site"			
"		Maximum flow	0.133	c.m/sec"	
"		Hydrograph volume	572.591	c.m"	
"	0.161	0.236	0.129	0.133"	
" 38		START/RE-START TOTALS 100"			
"	3	Runoff Totals on EXIT"			
"		Total Catchment area	2.060	hectare"	
"		Total Impervious area	0.699	hectare"	
"		Total % impervious	33.908"		
" 19		EXIT"			

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        \\geiconsultants.com\data\Data_Storage\"
"          Working\JAMES KEATING CONSTR\2404979 - 416149 Ross Property - 191
South Street, Elora\Design Phase\Design Calcs\Modelling Files\2025-10-27"
"          Output filename:                      Post_25yr.out"
"          Licensee name:                      "
"          Company                      "
"          Date & Time last used:                10/29/2025 at 1:26:55 PM"
" 31          TIME PARAMETERS"
"          5.000  Time Step"
"          240.000  Max. Storm length"
"          1500.000  Max. Hydrograph"
" 32          STORM Chicago storm"
"          1  Chicago storm"
"          746.059  Coefficient A"
"          0.085  Constant B"
"          0.692  Exponent C"
"          0.375  Fraction R"
"          240.000  Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          175.654  mm/hr"
"          Total depth          67.239  mm"
"          6  025hyd  Hydrograph extension used in this file"
" 33          CATCHMENT 200"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          200  Catchment 200"
"          45.000  % Impervious"
"          0.070  Total Area"
"          7.000  Flow length"
"          4.000  Overland Slope"
"          0.038  Pervious Area"
"          7.000  Pervious length"
"          4.000  Pervious slope"
"          0.032  Impervious Area"
"          7.000  Impervious length"
"          4.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.050  Impervious Lag constant (hours)"

```

"	1.500	Impervious Depression storage"				
"		0.022	0.000	0.000	0.000 c.m/sec"	
"		Catchment 200	Pervious	Impervious	Total Area	"
"		Surface Area	0.038	0.032	0.070	hectare"
"		Time of concentration	3.442	0.601	1.375	minutes"
"		Time to Centroid	100.486	111.156	108.249	minutes"
"		Rainfall depth	67.239	67.239	67.239	mm"
"		Rainfall volume	25.89	21.18	47.07	c.m"
"		Rainfall losses	48.411	5.774	29.224	mm"
"		Runoff depth	18.828	61.465	38.015	mm"
"		Runoff volume	7.25	19.36	26.61	c.m"
"		Runoff coefficient	0.280	0.914	0.565	"
"		Maximum flow	0.009	0.013	0.022	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.022	0.022	0.000	0.000"	
" 33		CATCHMENT 300"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	300	Catchment 300"				
"	0.000	% Impervious"				
"	0.050	Total Area"				
"	10.000	Flow length"				
"	10.000	Overland Slope"				
"	0.050	Pervious Area"				
"	10.000	Pervious length"				
"	10.000	Pervious slope"				
"	0.000	Impervious Area"				
"	10.000	Impervious length"				
"	10.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	12.500	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.012	0.022	0.000	0.000 c.m/sec"	
"		Catchment 300	Pervious	Impervious	Total Area	"
"		Surface Area	0.050	0.000	0.050	hectare"
"		Time of concentration	3.239	0.565	3.239	minutes"
"		Time to Centroid	100.274	111.177	100.274	minutes"
"		Rainfall depth	67.239	67.239	67.239	mm"
"		Rainfall volume	33.62	0.00	33.62	c.m"
"		Rainfall losses	48.248	6.186	48.248	mm"
"		Runoff depth	18.991	61.053	18.991	mm"

"	Runoff volume	9.50	0.00	9.50	c.m"
"	Runoff coefficient	0.282	0.000	0.282	"
"	Maximum flow	0.012	0.000	0.012	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.012	0.035	0.000	0.000"	
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.012	0.035	0.035	0.000"	
" 40	HYDROGRAPH Combine 1"				
"	6 Combine "				
"	1 Node #"				
"	Total Outflow from Site"				
"	Maximum flow	0.035		c.m/sec"	
"	Hydrograph volume	36.106		c.m"	
"	0.012	0.035	0.035	0.035"	
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"	0.012	0.000	0.035	0.035"	
" 33	CATCHMENT 400"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	400 Catchment 400"				
"	20.000 % Impervious"				
"	1.320 Total Area"				
"	300.000 Flow length"				
"	2.500 Overland Slope"				
"	1.056 Pervious Area"				
"	300.000 Pervious length"				
"	2.500 Pervious slope"				
"	0.264 Impervious Area"				
"	300.000 Impervious length"				
"	2.500 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	75.000 Pervious Max.infiltration"				
"	12.500 Pervious Min.infiltration"				
"	0.250 Pervious Lag constant (hours)"				
"	5.000 Pervious Depression storage"				
"	0.015 Impervious Manning 'n'"				
"	0.000 Impervious Max.infiltration"				
"	0.000 Impervious Min.infiltration"				
"	0.050 Impervious Lag constant (hours)"				
"	1.500 Impervious Depression storage"				
"	0.128	0.000	0.035	0.035 c.m/sec"	
"	Catchment 400	Pervious	Impervious	Total Area	"
"	Surface Area	1.056	0.264	1.320	hectare"
"	Time of concentration	37.783	6.593	23.596	minutes"
"	Time to Centroid	138.702	120.175	130.274	minutes"
"	Rainfall depth	67.239	67.239	67.239	mm"

"	Rainfall volume	710.04	177.51	887.55	c.m"
"	Rainfall losses	47.710	2.058	38.580	mm"
"	Runoff depth	19.529	65.181	28.659	mm"
"	Runoff volume	206.22	172.08	378.30	c.m"
"	Runoff coefficient	0.290	0.969	0.426	"
"	Maximum flow	0.072	0.105	0.128	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.128	0.128	0.035	0.035"
" 33	CATCHMENT 100"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	100 Catchment 100"				
"	65.000 % Impervious"				
"	0.620 Total Area"				
"	50.000 Flow length"				
"	3.000 Overland Slope"				
"	0.217 Pervious Area"				
"	50.000 Pervious length"				
"	3.000 Pervious slope"				
"	0.403 Impervious Area"				
"	50.000 Impervious length"				
"	3.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	75.000 Pervious Max.infiltration"				
"	12.500 Pervious Min.infiltration"				
"	0.250 Pervious Lag constant (hours)"				
"	5.000 Pervious Depression storage"				
"	0.015 Impervious Manning 'n'"				
"	0.000 Impervious Max.infiltration"				
"	0.000 Impervious Min.infiltration"				
"	0.050 Impervious Lag constant (hours)"				
"	1.500 Impervious Depression storage"				
"		0.192	0.128	0.035	0.035 c.m/sec"
"	Catchment 100	Pervious	Impervious	Total Area	"
"	Surface Area	0.217	0.403	0.620	hectare"
"	Time of concentration	12.208	2.130	3.530	minutes"
"	Time to Centroid	110.342	112.890	112.536	minutes"
"	Rainfall depth	67.239	67.239	67.239	mm"
"	Rainfall volume	145.91	270.97	416.88	c.m"
"	Rainfall losses	47.742	2.120	18.088	mm"
"	Runoff depth	19.497	65.119	49.151	mm"
"	Runoff volume	42.31	262.43	304.74	c.m"
"	Runoff coefficient	0.290	0.968	0.731	"
"	Maximum flow	0.035	0.181	0.192	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.192	0.286	0.035	0.035"
" 54	POND DESIGN"				

"	0.286	Current peak flow	c.m/sec"		
"	0.270	Target outflow	c.m/sec"		
"	683.0	Hydrograph volume	c.m"		
"	10.	Number of stages"			
"	392.080	Minimum water level	metre"		
"	393.950	Maximum water level	metre"		
"	392.080	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	389.240	0.000	0.000"		
"	389.440	0.02600	27.180"		
"	389.640	0.04200	54.360"		
"	389.840	0.05400	81.530"		
"	390.040	0.06400	108.710"		
"	390.240	0.1170	135.890"		
"	390.440	0.2150	163.070"		
"	390.600	0.2640	184.810"		
"	391.000	0.3560	184.950"		
"	391.100	0.6170	185.470"		
"		Peak outflow	0.184	c.m/sec"	
"		Maximum level	390.382	metre"	
"		Maximum storage	155.164	c.m"	
"		Centroidal lag	2.372	hours"	
"	0.192	0.286	0.184	0.035 c.m/sec"	
" 40		HYDROGRAPH Combine	1"		
"	6	Combine "			
"	1	Node #"			
"		Total Outflow from Site"			
"		Maximum flow	0.189	c.m/sec"	
"		Hydrograph volume	719.820	c.m"	
"	0.192	0.286	0.184	0.189"	
" 38		START/RE-START TOTALS 100"			
"	3	Runoff Totals on EXIT"			
"		Total Catchment area	2.060	hectare"	
"		Total Impervious area	0.699	hectare"	
"		Total % impervious	33.908"		
" 19		EXIT"			

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        \\geiconsultants.com\data\Data_Storage\"
"          Working\JAMES KEATING CONSTR\2404979 - 416149 Ross Property - 191
South Street, Elora\Design Phase\Design Calcs\Modelling Files\2025-10-27"
"          Output filename:                    Post_50yr.out"
"          Licensee name:                      "
"          Company                            "
"          Date & Time last used:              10/29/2025 at 1:27:32 PM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          240.000 Max. Storm length"
"          1500.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          820.361 Coefficient A"
"          0.010  Constant B"
"          0.691  Exponent C"
"          0.375  Fraction R"
"          240.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          194.803  mm/hr"
"          Total depth                74.358  mm"
"          6  050hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 200"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          200  Catchment 200"
"          45.000 % Impervious"
"          0.070  Total Area"
"          7.000  Flow length"
"          4.000  Overland Slope"
"          0.038  Pervious Area"
"          7.000  Pervious length"
"          4.000  Pervious slope"
"          0.032  Impervious Area"
"          7.000  Impervious length"
"          4.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.050  Impervious Lag constant (hours)"

```

"	1.500	Impervious Depression storage"				
"		0.026	0.000	0.000	0.000 c.m/sec"	
"		Catchment 200	Pervious	Impervious	Total Area	"
"		Surface Area	0.038	0.032	0.070	hectare"
"		Time of concentration	3.268	0.576	1.387	minutes"
"		Time to Centroid	101.060	110.975	107.990	minutes"
"		Rainfall depth	74.358	74.358	74.358	mm"
"		Rainfall volume	28.63	23.42	52.05	c.m"
"		Rainfall losses	50.449	6.524	30.683	mm"
"		Runoff depth	23.908	67.834	43.675	mm"
"		Runoff volume	9.20	21.37	30.57	c.m"
"		Runoff coefficient	0.322	0.912	0.587	"
"		Maximum flow	0.012	0.015	0.026	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.026	0.026	0.000	0.000"	
" 33		CATCHMENT 300"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	300	Catchment 300"				
"	0.000	% Impervious"				
"	0.050	Total Area"				
"	10.000	Flow length"				
"	10.000	Overland Slope"				
"	0.050	Pervious Area"				
"	10.000	Pervious length"				
"	10.000	Pervious slope"				
"	0.000	Impervious Area"				
"	10.000	Impervious length"				
"	10.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	12.500	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.016	0.026	0.000	0.000 c.m/sec"	
"		Catchment 300	Pervious	Impervious	Total Area	"
"		Surface Area	0.050	0.000	0.050	hectare"
"		Time of concentration	3.075	0.542	3.075	minutes"
"		Time to Centroid	100.863	110.989	100.863	minutes"
"		Rainfall depth	74.358	74.358	74.358	mm"
"		Rainfall volume	37.18	0.00	37.18	c.m"
"		Rainfall losses	50.290	7.006	50.290	mm"
"		Runoff depth	24.067	67.352	24.068	mm"

"	Runoff volume	12.03	0.00	12.03	c.m"
"	Runoff coefficient	0.324	0.000	0.324	"
"	Maximum flow	0.016	0.000	0.016	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.016	0.042	0.000	0.000"	
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.016	0.042	0.042	0.000"	
" 40	HYDROGRAPH Combine 1"				
"	6 Combine "				
"	1 Node #"				
"	Total Outflow from Site"				
"	Maximum flow	0.042		c.m/sec"	
"	Hydrograph volume	42.606		c.m"	
"	0.016	0.042	0.042	0.042"	
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"	0.016	0.000	0.042	0.042"	
" 33	CATCHMENT 400"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	400 Catchment 400"				
"	20.000 % Impervious"				
"	1.320 Total Area"				
"	300.000 Flow length"				
"	2.500 Overland Slope"				
"	1.056 Pervious Area"				
"	300.000 Pervious length"				
"	2.500 Pervious slope"				
"	0.264 Impervious Area"				
"	300.000 Impervious length"				
"	2.500 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	75.000 Pervious Max.infiltration"				
"	12.500 Pervious Min.infiltration"				
"	0.250 Pervious Lag constant (hours)"				
"	5.000 Pervious Depression storage"				
"	0.015 Impervious Manning 'n'"				
"	0.000 Impervious Max.infiltration"				
"	0.000 Impervious Min.infiltration"				
"	0.050 Impervious Lag constant (hours)"				
"	1.500 Impervious Depression storage"				
"	0.146	0.000	0.042	0.042 c.m/sec"	
"	Catchment 400	Pervious	Impervious	Total Area	"
"	Surface Area	1.056	0.264	1.320	hectare"
"	Time of concentration	35.872	6.326	23.352	minutes"
"	Time to Centroid	138.555	119.576	130.513	minutes"
"	Rainfall depth	74.358	74.358	74.358	mm"

"	Rainfall volume	785.22	196.30	981.52	c.m"
"	Rainfall losses	49.791	2.095	40.252	mm"
"	Runoff depth	24.567	72.262	34.106	mm"
"	Runoff volume	259.43	190.77	450.20	c.m"
"	Runoff coefficient	0.330	0.972	0.459	"
"	Maximum flow	0.091	0.116	0.146	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.146	0.146	0.042	0.042"
" 33	CATCHMENT 100"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	100 Catchment 100"				
"	65.000 % Impervious"				
"	0.620 Total Area"				
"	50.000 Flow length"				
"	3.000 Overland Slope"				
"	0.217 Pervious Area"				
"	50.000 Pervious length"				
"	3.000 Pervious slope"				
"	0.403 Impervious Area"				
"	50.000 Impervious length"				
"	3.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	75.000 Pervious Max.infiltration"				
"	12.500 Pervious Min.infiltration"				
"	0.250 Pervious Lag constant (hours)"				
"	5.000 Pervious Depression storage"				
"	0.015 Impervious Manning 'n'"				
"	0.000 Impervious Max.infiltration"				
"	0.000 Impervious Min.infiltration"				
"	0.050 Impervious Lag constant (hours)"				
"	1.500 Impervious Depression storage"				
"		0.217	0.146	0.042	0.042 c.m/sec"
"	Catchment 100	Pervious	Impervious	Total Area	"
"	Surface Area	0.217	0.403	0.620	hectare"
"	Time of concentration	11.591	2.044	3.524	minutes"
"	Time to Centroid	110.732	112.561	112.278	minutes"
"	Rainfall depth	74.358	74.358	74.358	mm"
"	Rainfall volume	161.36	299.66	461.02	c.m"
"	Rainfall losses	49.795	2.259	18.896	mm"
"	Runoff depth	24.563	72.099	55.461	mm"
"	Runoff volume	53.30	290.56	343.86	c.m"
"	Runoff coefficient	0.330	0.970	0.746	"
"	Maximum flow	0.043	0.201	0.217	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.217	0.325	0.042	0.042"
" 54	POND DESIGN"				

"	0.325	Current peak flow	c.m/sec"		
"	0.270	Target outflow	c.m/sec"		
"	794.1	Hydrograph volume	c.m"		
"	10.	Number of stages"			
"	392.080	Minimum water level	metre"		
"	393.950	Maximum water level	metre"		
"	392.080	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	389.240	0.000	0.000"		
"	389.440	0.02600	27.180"		
"	389.640	0.04200	54.360"		
"	389.840	0.05400	81.530"		
"	390.040	0.06400	108.710"		
"	390.240	0.1170	135.890"		
"	390.440	0.2150	163.070"		
"	390.600	0.2640	184.810"		
"	391.000	0.3560	184.950"		
"	391.100	0.6170	185.470"		
"		Peak outflow	0.220	c.m/sec"	
"		Maximum level	390.469	metre"	
"		Maximum storage	166.993	c.m"	
"		Centroidal lag	2.358	hours"	
"	0.217	0.325	0.220	0.042	c.m/sec"
" 40		HYDROGRAPH Combine	1"		
"	6	Combine "			
"	1	Node #"			
"		Total Outflow from Site"			
"		Maximum flow	0.227	c.m/sec"	
"		Hydrograph volume	835.771	c.m"	
"	0.217	0.325	0.220	0.227"	
" 38		START/RE-START TOTALS 100"			
"	3	Runoff Totals on EXIT"			
"		Total Catchment area	2.060	hectare"	
"		Total Impervious area	0.699	hectare"	
"		Total % impervious	33.908"		
" 19		EXIT"			

```

"          MIDUSS Output ----->"
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"          MIDUSS created                      Sunday, February 7, 2010"
"          10  Units used:                      ie METRIC"
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South Street, Elora\Design Phase\Design Calcs\Modelling Files\2025-10-27"
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" 31          TIME PARAMETERS"
"          3.000  Time Step"
"          240.000  Max. Storm length"
"          1500.000  Max. Hydrograph"
" 32          STORM Chicago storm"
"          1  Chicago storm"
"          901.088  Coefficient A"
"          0.043  Constant B"
"          0.692  Exponent C"
"          0.375  Fraction R"
"          240.000  Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          302.460  mm/hr"
"          Total depth          81.221  mm"
"          6  100hyd  Hydrograph extension used in this file"
" 33          CATCHMENT 200"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          200  Catchment 200"
"          45.000  % Impervious"
"          0.070  Total Area"
"          7.000  Flow length"
"          4.000  Overland Slope"
"          0.038  Pervious Area"
"          7.000  Pervious length"
"          4.000  Pervious slope"
"          0.032  Impervious Area"
"          7.000  Impervious length"
"          4.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          12.500  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.050  Impervious Lag constant (hours)"

```

"	1.500	Impervious Depression storage"				
"		0.041	0.000	0.000	0.000 c.m/sec"	
"		Catchment 200	Pervious	Impervious	Total Area	"
"		Surface Area	0.038	0.032	0.070	hectare"
"		Time of concentration	2.690	0.483	1.192	minutes"
"		Time to Centroid	100.076	108.662	105.905	minutes"
"		Rainfall depth	81.221	81.221	81.221	mm"
"		Rainfall volume	31.27	25.58	56.85	c.m"
"		Rainfall losses	51.766	5.093	30.763	mm"
"		Runoff depth	29.455	76.128	50.458	mm"
"		Runoff volume	11.34	23.98	35.32	c.m"
"		Runoff coefficient	0.363	0.937	0.621	"
"		Maximum flow	0.020	0.024	0.041	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.041	0.041	0.000	0.000"	
" 33		CATCHMENT 300"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	300	Catchment 300"				
"	0.000	% Impervious"				
"	0.050	Total Area"				
"	10.000	Flow length"				
"	10.000	Overland Slope"				
"	0.050	Pervious Area"				
"	10.000	Pervious length"				
"	10.000	Pervious slope"				
"	0.000	Impervious Area"				
"	10.000	Impervious length"				
"	10.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	12.500	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.025	0.041	0.000	0.000 c.m/sec"	
"		Catchment 300	Pervious	Impervious	Total Area	"
"		Surface Area	0.050	0.000	0.050	hectare"
"		Time of concentration	2.531	0.455	2.531	minutes"
"		Time to Centroid	99.871	108.742	99.871	minutes"
"		Rainfall depth	81.221	81.221	81.221	mm"
"		Rainfall volume	40.61	0.00	40.61	c.m"
"		Rainfall losses	51.886	5.171	51.886	mm"
"		Runoff depth	29.335	76.049	29.335	mm"

"	Runoff volume	14.67	0.00	14.67	c.m"
"	Runoff coefficient	0.361	0.000	0.361	"
"	Maximum flow	0.025	0.000	0.025	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.025	0.065	0.000	0.000"	
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.025	0.065	0.065	0.000"	
" 40	HYDROGRAPH Combine 1"				
"	6 Combine "				
"	1 Node #"				
"	Total Outflow from Site"				
"	Maximum flow	0.065		c.m/sec"	
"	Hydrograph volume	49.988		c.m"	
"	0.025	0.065	0.065	0.065"	
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"	0.025	0.000	0.065	0.065"	
" 33	CATCHMENT 400"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	400 Catchment 400"				
"	20.000 % Impervious"				
"	1.320 Total Area"				
"	300.000 Flow length"				
"	2.500 Overland Slope"				
"	1.056 Pervious Area"				
"	300.000 Pervious length"				
"	2.500 Pervious slope"				
"	0.264 Impervious Area"				
"	300.000 Impervious length"				
"	2.500 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	75.000 Pervious Max.infiltration"				
"	12.500 Pervious Min.infiltration"				
"	0.250 Pervious Lag constant (hours)"				
"	5.000 Pervious Depression storage"				
"	0.015 Impervious Manning 'n'"				
"	0.000 Impervious Max.infiltration"				
"	0.000 Impervious Min.infiltration"				
"	0.050 Impervious Lag constant (hours)"				
"	1.500 Impervious Depression storage"				
"	0.197	0.000	0.065	0.065 c.m/sec"	
"	Catchment 400	Pervious	Impervious	Total Area	"
"	Surface Area	1.056	0.264	1.320	hectare"
"	Time of concentration	29.529	5.305	19.825	minutes"
"	Time to Centroid	134.559	117.611	127.770	minutes"
"	Rainfall depth	81.221	81.221	81.221	mm"

"	Rainfall volume	857.69	214.42	1072.11	c.m"
"	Rainfall losses	51.489	1.743	41.540	mm"
"	Runoff depth	29.732	79.478	39.681	mm"
"	Runoff volume	313.97	209.82	523.79	c.m"
"	Runoff coefficient	0.366	0.979	0.489	"
"	Maximum flow	0.130	0.165	0.197	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.197	0.197	0.065	0.065"
" 33	CATCHMENT 100"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	100 Catchment 100"				
"	65.000 % Impervious"				
"	0.620 Total Area"				
"	50.000 Flow length"				
"	3.000 Overland Slope"				
"	0.217 Pervious Area"				
"	50.000 Pervious length"				
"	3.000 Pervious slope"				
"	0.403 Impervious Area"				
"	50.000 Impervious length"				
"	3.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	75.000 Pervious Max.infiltration"				
"	12.500 Pervious Min.infiltration"				
"	0.250 Pervious Lag constant (hours)"				
"	5.000 Pervious Depression storage"				
"	0.015 Impervious Manning 'n'"				
"	0.000 Impervious Max.infiltration"				
"	0.000 Impervious Min.infiltration"				
"	0.050 Impervious Lag constant (hours)"				
"	1.500 Impervious Depression storage"				
"		0.322	0.197	0.065	0.065 c.m/sec"
"	Catchment 100	Pervious	Impervious	Total Area	"
"	Surface Area	0.217	0.403	0.620	hectare"
"	Time of concentration	9.541	1.714	3.034	minutes"
"	Time to Centroid	108.871	111.074	110.702	minutes"
"	Rainfall depth	81.221	81.221	81.221	mm"
"	Rainfall volume	176.25	327.32	503.57	c.m"
"	Rainfall losses	51.613	2.651	19.788	mm"
"	Runoff depth	29.608	78.570	61.433	mm"
"	Runoff volume	64.25	316.64	380.88	c.m"
"	Runoff coefficient	0.365	0.967	0.756	"
"	Maximum flow	0.071	0.303	0.322	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.322	0.451	0.065	0.065"
" 54	POND DESIGN"				

"	0.451	Current peak flow	c.m/sec"		
"	0.270	Target outflow	c.m/sec"		
"	904.7	Hydrograph volume	c.m"		
"	10.	Number of stages"			
"	392.080	Minimum water level	metre"		
"	393.950	Maximum water level	metre"		
"	392.080	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	389.240	0.000	0.000"		
"	389.440	0.02600	27.180"		
"	389.640	0.04200	54.360"		
"	389.840	0.05400	81.530"		
"	390.040	0.06400	108.710"		
"	390.240	0.1170	135.890"		
"	390.440	0.2150	163.070"		
"	390.600	0.2640	184.810"		
"	391.000	0.3560	184.950"		
"	391.100	0.6170	185.470"		
"		Peak outflow	0.261	c.m/sec"	
"		Maximum level	390.596	metre"	
"		Maximum storage	184.204	c.m"	
"		Centroidal lag	2.302	hours"	
"	0.322	0.451	0.261	0.065 c.m/sec"	
" 40		HYDROGRAPH Combine	1"		
"	6	Combine "			
"	1	Node #"			
"		Total Outflow from Site"			
"		Maximum flow	0.276	c.m/sec"	
"		Hydrograph volume	954.918	c.m"	
"	0.322	0.451	0.261	0.276"	
" 38		START/RE-START TOTALS 100"			
"	3	Runoff Totals on EXIT"			
"		Total Catchment area	2.060	hectare"	
"		Total Impervious area	0.699	hectare"	
"		Total % impervious	33.908"		
" 19		EXIT"			

Appendix D Oil/Grit Separator Sizing Results

Stormceptor® EF Sizing Report

Imbrium® Systems

ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION

09/04/2025

Province:	Ontario	Project Name:	191 Wellington Rd 7 and 290 South St
City:	Township of Centre Wellington (Elora)	Project Number:	2404979
Nearest Rainfall Station:	WATERLOO WELLINGTON AP	Designer Name:	Sabrina Jivani
Climate Station Id:	6149387	Designer Company:	GEI
Years of Rainfall Data:	34	Designer Email:	sjivani@geiconsultants.com
Site Name:		Designer Phone:	416-689-7699
		EOR Name:	
		EOR Company:	
		EOR Email:	
		EOR Phone:	

Drainage Area (ha):	2.01
% Imperviousness:	35.00

Runoff Coefficient 'c': 0.51

Particle Size Distribution:	Fine
Target TSS Removal (%):	80.0

Required Water Quality Runoff Volume Capture (%):	90.00
Estimated Water Quality Flow Rate (L/s):	38.84
Oil / Fuel Spill Risk Site?	Yes
Upstream Flow Control?	Yes
Upstream Orifice Control Flow Rate to Stormceptor (L/s):	261.00
Peak Conveyance (maximum) Flow Rate (L/s):	
Influent TSS Concentration (mg/L):	200
Estimated Average Annual Sediment Load (kg/yr):	807
Estimated Average Annual Sediment Volume (L/yr):	656

**Net Annual Sediment
(TSS) Load Reduction
Sizing Summary**

Stormceptor Model	TSS Removal Provided (%)
EFO4	69
EFO5	76
EFO6	82
EFO8	89
EFO10	93
EFO12	96

Recommended Stormceptor EFO Model: EFO6
Estimated Net Annual Sediment (TSS) Load Reduction (%): 82
Water Quality Runoff Volume Capture (%): > 90

Stormceptor® EF Sizing Report

THIRD-PARTY TESTING AND VERIFICATION

► **Stormceptor® EF and Stormceptor® EFO** are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** and performance has been third-party verified in accordance with the **ISO 14034 Environmental Technology Verification (ETV)** protocol.

PERFORMANCE

► **Stormceptor® EF and EFO** remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

PARTICLE SIZE DISTRIBUTION (PSD)

► The **Canadian ETV PSD** shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle Size (µm)	Percent Less Than	Particle Size Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5

Stormceptor®EF Sizing Report

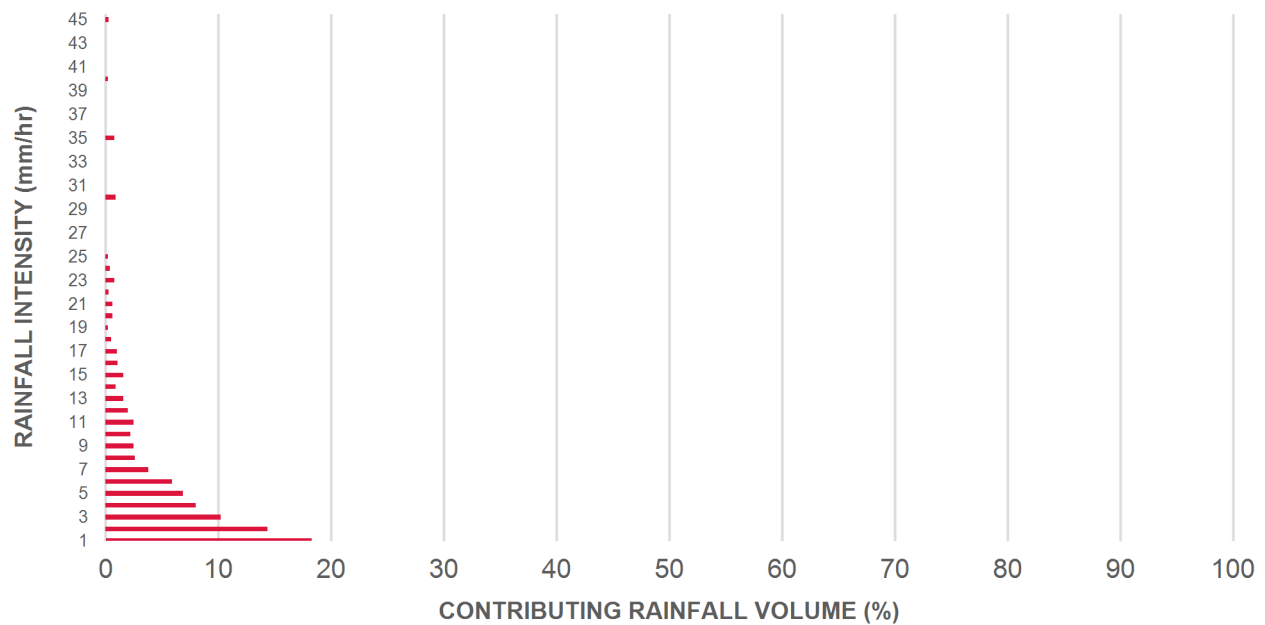
Upstream Flow Controlled Results

Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m²)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.50	8.5	8.5	1.42	85.0	33.0	100	8.5	8.5
1.00	18.3	26.8	2.85	171.0	65.0	100	18.3	26.8
2.00	14.4	41.3	5.70	342.0	130.0	92	13.3	40.1
3.00	10.2	51.5	8.55	513.0	195.0	84	8.6	48.7
4.00	8.0	59.5	11.40	684.0	260.0	80	6.4	55.2
5.00	6.9	66.4	14.25	855.0	325.0	78	5.4	60.5
6.00	5.9	72.3	17.10	1026.0	390.0	74	4.4	64.9
7.00	3.8	76.1	19.95	1197.0	455.0	72	2.7	67.6
8.00	2.6	78.7	22.80	1368.0	520.0	68	1.8	69.4
9.00	2.5	81.1	25.65	1539.0	585.0	66	1.6	71.0
10.00	2.2	83.3	28.50	1710.0	650.0	64	1.4	72.4
11.00	2.5	85.8	31.35	1881.0	715.0	64	1.6	74.0
12.00	2.0	87.8	34.20	2052.0	780.0	63	1.3	75.2
13.00	1.6	89.4	37.05	2223.0	845.0	63	1.0	76.3
14.00	0.9	90.4	39.90	2394.0	910.0	62	0.6	76.8
15.00	1.6	91.9	42.75	2565.0	975.0	62	1.0	77.8
16.00	1.1	93.0	45.60	2736.0	1040.0	61	0.7	78.5
17.00	1.0	94.0	48.45	2907.0	1105.0	59	0.6	79.1
18.00	0.5	94.6	51.30	3078.0	1170.0	58	0.3	79.4
19.00	0.2	94.8	54.15	3249.0	1235.0	56	0.1	79.5
20.00	0.6	95.4	57.00	3420.0	1300.0	55	0.3	79.9
21.00	0.6	96.1	59.85	3591.0	1365.0	53	0.3	80.2
22.00	0.3	96.4	62.70	3762.0	1430.0	51	0.1	80.4
23.00	0.8	97.2	65.54	3933.0	1495.0	49	0.4	80.8
24.00	0.4	97.6	68.39	4104.0	1560.0	47	0.2	81.0
25.00	0.2	97.8	71.24	4275.0	1625.0	45	0.1	81.0
30.00	0.9	98.7	85.49	5130.0	1950.0	38	0.3	81.4
35.00	0.8	99.5	99.74	5985.0	2275.0	32	0.3	81.6
40.00	0.2	99.7	113.99	6839.0	2601.0	28	0.1	81.7
45.00	0.3	100.0	128.24	7694.0	2926.0	25	0.1	81.8
Estimated Net Annual Sediment (TSS) Load Reduction =								82 %

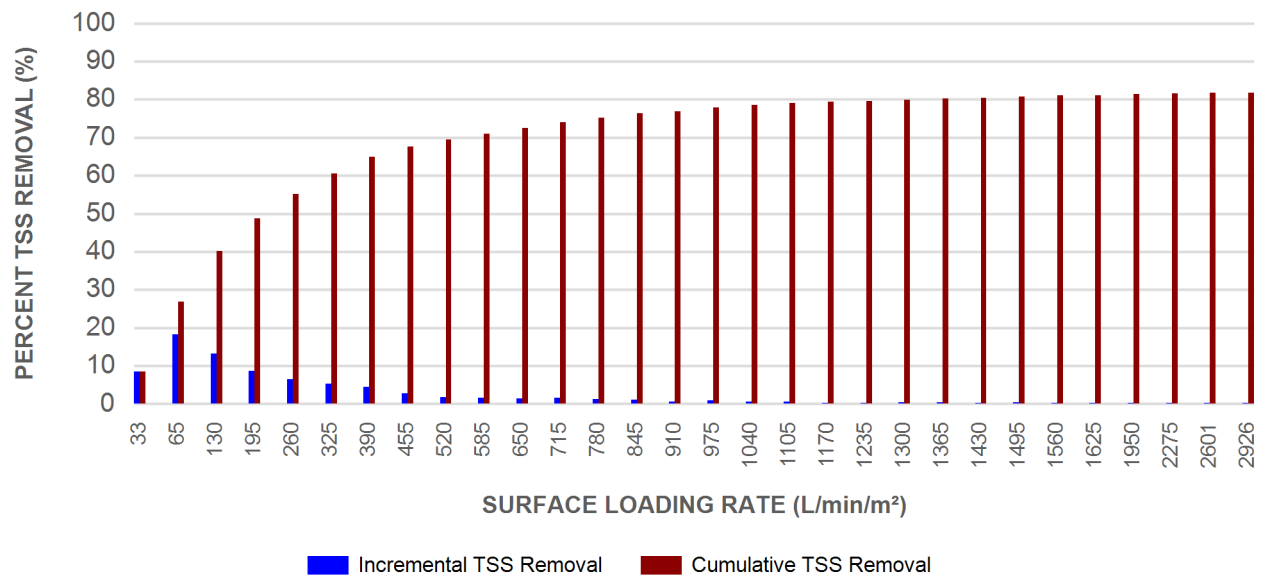
Climate Station ID: 6149387 Years of Rainfall Data: 34

Stormceptor®EF Sizing Report

RAINFALL DATA FROM WATERLOO WELLINGTON AP RAINFALL STATION



INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



Stormceptor® EF Sizing Report

Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF5 / EFO5	1.5	5	90	762	30	762	30	710	25
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

SCOUR PREVENTION AND ONLINE CONFIGURATION

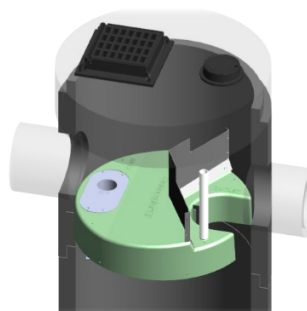
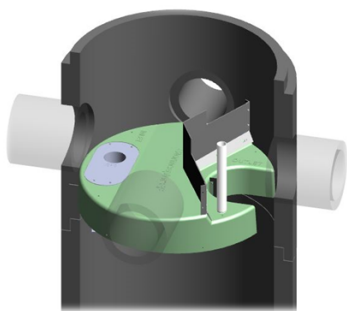
► **Stormceptor® EF and EFO** feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

DESIGN FLEXIBILITY

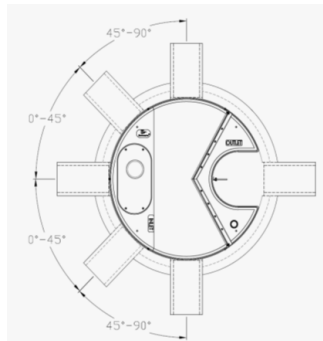
► **Stormceptor® EF and EFO** offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

OIL CAPTURE AND RETENTION

► While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, **Stormceptor® EFO** has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid re-entrainment testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.



Stormceptor® EF Sizing Report



INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1.

For submerged conditions the applicable K value is 3.0.

Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF5 / EFO5	1.5	5	1.62	5.3	420	111	305	10	2124	75	2612	5758
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

*Increased sump depth may be added to increase sediment storage capacity

** Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³)

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

STANDARD STORMCEPTOR EF/EFO SPECIFICATION

For specifications, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

STANDARD PERFORMANCE SPECIFICATION FOR “OIL GRIT SEPARATOR” (OGS) STORMWATER QUALITY TREATMENT DEVICE

PART 1 – GENERAL

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program's **Procedure for Laboratory Testing of Oil-Grit Separators**

1.3 SUBMITTALS

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

PART 2 – PRODUCTS

2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The minimum sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m ³ sediment / 265 L oil
	5 ft (1524 mm) Diameter OGS Units:	1.95 m ³ sediment / 420 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m ³ sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m ³ sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m ³ sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m ³ sediment / 2,476 L oil

PART 3 – PERFORMANCE & DESIGN

Stormceptor®EF Sizing Report

3.1 GENERAL

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m² to 1400 L/min/m², and as stated in the ISO 14034 ETV Verification Statement for the OGS device.

3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m² and 1400 L/min/m² shall be based on linear interpolation of data between consecutive tested surface loading rates.

3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m² shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m². No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m².

3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m² shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m², and shall be calculated using a simple proportioning formula, with 1400 L/min/m² in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m².

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m².

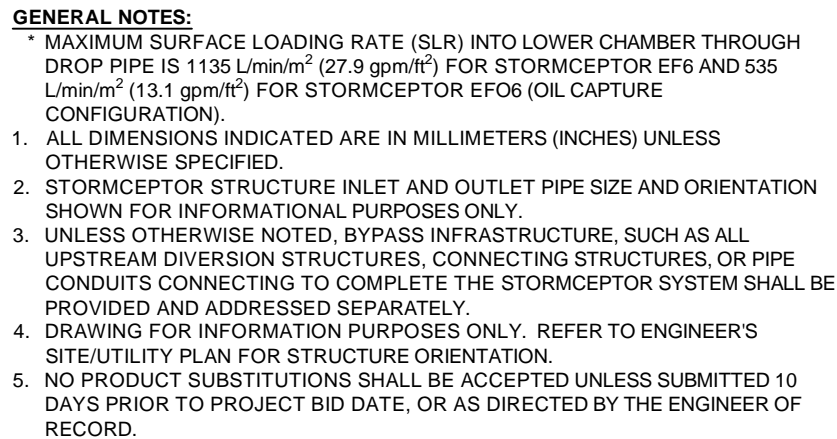
3.4 LIGHT LIQUID RE-ENTRAINMENT SIMULATION TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of completed third-party Light Liquid

Stormceptor®**EF** Sizing Report

Re-entrainment Simulation Testing in accordance with the Canadian ETV **Program's Procedure for Laboratory Testing of Oil-Grit Separators**, with results reported within the Canadian ETV or ISO 14034 ETV verification. This re-entrainment testing is conducted with the device pre-loaded with low density polyethylene (LDPE) plastic beads as a surrogate for light liquids such as oil and fuel. Testing is conducted on the same OGS unit tested for sediment removal to assess whether light liquids captured after a spill are effectively retained at high flow rates.

3.4.1 For an OGS device to be an acceptable stormwater treatment device on a site where vehicular traffic occurs and the potential for an oil or fuel spill exists, the OGS device must have reported verified performance results of greater than 99% cumulative retention of LDPE plastic beads for the five specified surface loading rates (ranging 200 L/min/m² to 2600 L/min/m²) in accordance with the Light Liquid Re-entrainment Simulation Testing within the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**. However, an OGS device shall not be allowed if the Light Liquid Re-entrainment Simulation Testing was performed with screening components within the OGS device that are effective at retaining the LDPE plastic beads, but would not be expected to retain light liquids such as oil and fuel.



FOR SITE SPECIFIC DRAWINGS PLEASE CONTACT YOUR LOCAL STORMCEPTOR REPRESENTATIVE. SITE SPECIFIC DRAWINGS ARE BASED ON THE BEST AVAILABLE INFORMATION AT THE TIME. SOME FIELD REVISIONS TO THE SYSTEM LOCATION OR CONNECTION PIPING MAY BE NECESSARY BASED ON AVAILABLE SPACE OR SITE CONFIGURATION REVISIONS. ELEVATIONS SHOULD BE MAINTAINED EXCEPT WHERE NOTED ON BYPASS STRUCTURE (IF REQUIRED).

- A. ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
- B. CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE STRUCTURE (LIFTING CLUTCHES PROVIDED)
- C. CONTRACTOR WILL INSTALL AND LEVEL THE STRUCTURE, SEALING THE JOINTS, LINE ENTRY AND EXIT POINTS (NON-SHRINK GROUT WITH APPROVED WATERSTOP OR FLEXIBLE BOOT)
- D. CONTRACTOR TO TAKE APPROPRIATE MEASURES TO PROTECT THE DEVICE FROM CONSTRUCTION-RELATED EROSION RUNOFF.
- E. DEVICE ACTIVATION, BY CONTRACTOR, SHALL OCCUR ONLY AFTER SITE HAS BEEN STABILIZED AND THE STORMCEPTOR UNIT IS CLEAN AND FREE OF DEBRIS.



<u>SITE SPECIFIC DATA REQUIREMENTS</u>					
STORMCEPTOR MODEL			EFO6		
STRUCTURE ID					*
HYDROCARBON STORAGE REQ'D (L)					*
WATER QUALITY FLOW RATE (L/s)					*
PEAK FLOW RATE (L/s)					*
RETURN PERIOD OF PEAK FLOW (yrs)					*
DRAINAGE AREA (HA)					*
DRAINAGE AREA IMPERVIOUSNESS (%)					*
PIPE DATA:	I.E.	MAT'L	DIA	SLOPE %	HGL
INLET #1	*	*	*	*	*
INLET #2	*	*	*	*	*
OUTLET	*	*	*	*	*
* PER ENGINEER OF RECORD					

Stormceptor® EF



imbrium®
407 FAIRVIEW DRIVE, WHITEY, ON L1N 3J1
F 900-565-4801 CA 416-860-9900 INTL. +1-416-860-9900

DATE: 10/13/2017	
DESIGNED: JSK	DRAWN: JSK
CHECKED: BSF	APPROVED:
PROJECT No.: EFO6	SEQUENCE No.: *
SHEET: 1 OF 1	

SCALE = NTS